 Northside Remodeling	Northside Remodeling Services License # GCLT-CO000297 3162 Dogwood Dr, Hapeville, Georgia 30354 Phone 404-444-8737			
	Estimate			
CUSTOMER:	Factum	RE:	New Construction	
CO Address:		Project Address:	641 Warren Ave	
Attn:	Manuella Villa	Est Date:	Exp Date:	10/29/22
Quote #:	2022-118	Ver #/Date:	10/27/22	

Subject to the terms and conditions stated in this agreement, but unless otherwise stated, all line items include labor and materials for the Scope of Work described below. Any materials or selections being provided by customer MUST be delivered to contractor or jobsite in a timely fashion or are subject to contractor default selections.

Scope of Work to be Performed:

Category	Description	Qty	Cost	Total	Notes
Permits	Permit Fee - Municipality	1	\$2,500	\$2,500	agreed to remove from this budget
Utilities	Porta Potty	5	\$150	\$750	
Silt Fence/Tree Fencing	Install silt fencing and tree fencing per site plan	1	\$2,200	\$2,200	
General Labor - Jobsite Clec	Weekly job site clean up and debris removal	1	\$1,500	\$1,500	
Landscaping	Remove 2 trees	2	\$2,500	\$5,000	
Demolition - Lot Clearing	Clear area for grading and dig footing	1	\$5,500	\$5,500	
Footings	Pour footings (labor and concrete)	1	\$7,500	\$7,500	
Foundation	Install block foundation and Piers	1	\$9,000	\$9,000	
Concrete	Pour garage slabs and sidewalks to front door	1	\$14,000	\$14,000	
Framing	Frame per plans	1	\$78,000	\$78,000	
Roofing	Install new architectural 30 year shingles on	1	\$7,500	\$7,500	
Roofing	Metal Roof on Front Porch	1	\$2,000	\$2,000	
Decking	Install decking and railing for deck and porch	1	\$11,000	\$11,000	
Window	Install new vinyl windows	1	\$23,000	\$23,000	
Siding	Install Siding Per Plans	1	\$14,000	\$14,000	
Exterior Trim	Exterior Trim, Soffit, Fascia	1	\$6,000	\$6,000	
Exterior Door	Install Exterior Doors Per plans	1	\$3,000	\$3,000	
Gutters	Install gutters and lot filtration flow wells	1	\$4,500	\$4,500	
Painting - Exterior	Paint foundation, siding, trim and paint/stain all decking	1	\$8,000	\$8,000	
Electrical	Wire Per Plans	1	\$17,000	\$17,000	
Electrical - Fixtures	Electrical fixture allowance	1	\$3,500	\$3,500	
Plumbing	Plumb per plans	1	\$15,000	\$15,000	
Plumbing - Fixtures	Plumbing fixture allowanace(shower valves,toilets,tubs,sinks and faucets)	1	\$4,500	\$4,500	
HVAC - Labor and Materials	Install 2 HVAC systems	1	\$12,000	\$12,000	
Insulation	New wall, floor and ceiling batt insulation throughout.	1	\$5,000	\$5,000	
Drywall	Hang, finish and sand drywall throughout	1	\$12,000	\$12,000	
Door/Trim	Install Interior Doors - Install Interior Baseboard, Door & Window Trim	1	\$12,000	\$12,000	
Painting - Interior	All interior walls, ceilings, doors and trim	1	\$9,000	\$9,000	
Flooring - Wood	Install wood in common areas and carpet in bedrooms	1	\$15,000	\$15,000	
Tiling	Tile all bathrooms. Average tile allowance \$4/sqft	1	\$15,000	\$15,000	
Cabinetry - Kitchen	New kitchen shaker cabinets and Island	1	\$16,000	\$16,000	
Countertops	Quartz countertop for Kitchen and Baths	1	\$8,000	\$8,000	
Backsplash	Backsplash for kitchen area	1	\$1,400	\$1,400	
Cabinetry - Bath	New bath vanities	1	\$2,500	\$2,500	
Shower Enclosure	Frameless shower enclosure for master	1	\$2,500	\$2,500	
Hardware Material	Allowance for Hardware - Door, Cabinet and Bath Trim	1	\$800	\$800	
Hardware labor	Install All Hardware - Door, Cabinet and Bath Trim	1	\$550	\$550	
Appliances	Stainless Steel Package (Allowance)	1	\$5,000	\$5,000	
Appliances Install	Install all appliances	1	\$500	\$500	
Landscaping	General landscaping -TBD	1	\$3,000	\$3,000	
Cleaning	Final Cleaning	1	\$700	\$700	
Total Estimate				\$365,900	

NEW CONSTRUCTION

641 WARREN AVE.
SCOTSDALE, GA 30079

SCOPE OF WORK:

New construction of 3/2.5 SFR on crawlspace,
with front porch and rear deck.

SQUARE FOOTAGE:

Proposed First Floor: 1124 sf
Proposed Second Floor: 997 sf
Proposed Porch: 100 sf
Proposed Deck: 171 sf

PROPOSED TOTAL: 2420 sf

27'-2 1/16"

Layout Page Table	
Label	Title
A0	Cover Sheet
A0.1	General Notes
A1.1	Proposed Elevations
A1.2	Proposed Elevations
A1.3	Proposed Floor Plan
A1.4	Proposed Floor Plan
A1.5	Proposed Floor Plan
S1.1	Foundation Plan
S1.2	Floor Framing
S1.3	Ceiling Framing
S1.4	Roof Framing
S1.5	Roof Plan
S1.6	Wall Sections
S1.7	Load Calculations
S1.8	Load Calculations
S1.9	Load Calculations
S1.10	Load Calculations
S1.11	Load Calculations
S1.12	Load Calculations
D1	Deck Framing
D2	Deck Framing

APPLICABLE CODES:

International Building Code, 2018 Edition, with Georgia Amendments (2020) (2022)
International Residential Code for One- and Two-Family Dwellings, 2018 Edition, with Georgia Amendments (2020)
International Fire Code, 2018 Edition, with Georgia Amendments (2020)
International Plumbing Code, 2018 Edition, with Georgia Amendments (2020) (2022)
International Mechanical Code, 2018 Edition, with Georgia Amendments (2020)
International Fuel Gas Code, 2018 Edition, with Georgia Amendments (2020) (2022)
National Electrical Code, 2020 Edition, with no Georgia Amendments (2021)
International Energy Conservation Code, 2015 Edition, with Georgia Supplements and Amendments (2020) (2022)
2018 NFPA 101 - Life Safety Code with State Amendments (2020)



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SCOTSDALE, GA 30079



DATE:

8/8/2022

SCALE:

SHEET:

A0

NEW CONSTRUCTION

641 WARREN AVE.
SCOTSDALE, GA 30079

CONSTRUCTION AND FRAMING NOTES

1. DESIGN LOADS ARE AS FOLLOWS PER SQ. FT.

LOCATION	LIVE	DEAD	DEFLECT LIMIT
1 ST FLOOR	40 LB	10 LB	L/360
2 ND FLOOR (SLEEPING AREA)	30 LB	10 LB	L/360
ATTIC (NON STORAGE)	10 LB	5LB	L/240
ATTIC (STORAGE)	20 LB	10 LB	L/240
ROOF (W/ FINISHED CEILING)	30 LB	15 LB	L/240
ROOF(NO FINISHED CEILING)	30 LB	7LB	L/180
DECKS	60 LB	10LB	L/360

SNOW LOADS HAVE BEEN ADJUSTED TO REFLECT THE SLIDE OFF FACTOR AS A FUNCTION OF ROOF PITCH. RAFTER SIZES MAY HAVE TO BE INCREASED TO ACCOMMODATE HIGHER SNOW LOADS. VERIFY WITH LOCAL CODES.

2. LUMBER SHALL BE DOUGLAS-FIR-LARCH, HEM-FIR, OR SOUTHERN YELLOW PINE WITH FB=1450 AND E=1.6 MINIMUM.

ALL PRESSURE TREATED LUMBER WILL BE A MINIMUM OF SYP#2 WITH A MOISTURE CONTENT OF 19%

3. ALL HEADERS SHALL BE FREE FROM ALL SPLITS, CHECKS, OR SHAKES.

4. UNLESS NOTED OTHERWISE, PROVIDE DOUBLE HEADER JOISTS AND TRIMMERS AT ALL FLOOR OPENINGS, DOUBLE JOISTS UNDER ALL PARALLEL PARTITIONS, DOUBLE 2X12 HEADERS WITH 1/2" PLYWOOD, GLUED BETWEEN AND NAILED, FOR ALL OPENINGS IN 2X6 WALLS. DOUBLE 2X12 HEADERS NAILED TOGETHER FOR ALL OPENINGS IN 2X4 WALLS.

5. FLOOR CONSTRUCTION: 3/4" TONGUE AND GROOVE SUBFLOOR WITH FINISH MATERIAL OVER.

6. STAIR CONSTRUCTION SHALL CONSIST OF (3) 2X2 STRINGERS, 5/4" OR 2X THICK TREADS AND 3/4" THICK RISERS OR MATERIALS FABRICATED BY A COMPONENT MANUFACTURER.

7. ALL WOOD PLATES IN CONTACT WITH CONCRETE SHALL BE PRESSURE TREATED AND SILICONE SEALED.

8. MICRO-LAM BEAMS SHALL HAVE BENDING STRESS: FB=2800 PSI. VERIFY WITH LOCAL CODES.

9. SPECIAL UPLIFT CONNECTORS AS INDICATED AT CANTILEVERED JOISTS SHALL BE SIMPSON STRONG TIE ANCHORS OR EQUAL.

10. MINIMUM HEADER SIZE SHALL BE (2) 2"x6" UNLESS NOTED OTHERWISE EXTERIOR WALLS SHALL BE (2) 2X12 WITH 1/2" PLYWOOD.

11. ALL STRUCTURAL STEEL SHALL CONFORM WITH ASTM SPECIFICATION A-36.

12. UNLESS OTHERWISE NOTED, PROVIDE A 2X PLATE BOLTED TO THE TOP FLANGE OF ALL STEEL BEAMS WITH 3/8" DIAMETER BOLTS STAGGERED AT 24" ON CENTER. RIGIDLY FASTEN ALL CONNECTING RAFTERS AND JOISTS AS APPROVED BY GOVERNING CODES, UNLESS OTHERWISE NOTED.

13. FLOOR FRAMING LAYOUT SHALL BE COORDINATED WITH THE GENERAL AND HVAC CONTRACTORS TO PROVIDE ACCESS CHASES AND UNOBSTRUCTED RUNS FOR HVAC DUCT WORK. FLOOR TRUSS LAYOUT TO BE ENGINEERED BY TRUSS MANUFACTURE.

14. PROVIDE BRIDGING OR BLOCKING AT MIDSPAN OF JOISTS/RAFTERS/TRUSSES. MAXIMUM SPACING BETWEEN BEARING WALL AND BLOCKING IS 8'0".

15. THESE FRAMING PLANS WERE DESIGNED USING STANDARD CONSTRUCTION PRACTICES. THEY CONFORM TO STANDARD BUILDING CODES. DUE TO VARIATIONS IN LOCAL CODES AND GEOLOGICAL CONDITIONS REVISIONS MAY BE REQUIRED TO THESE PLANS.

16. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE LOCAL CODES. REGULATIONS AND FHA/VA MPS. THE BUILDER SHALL VERIFY ALL CONDITIONS WITH LOCAL STRUCTURAL ENGINEERS AND CODE OFFICIALS PRIOR TO USING THE FRAMING MATERIALSPROVIDED TO INSURE COMPLIANCE WITH CODES AND STRUCTURAL INTEGRITY.

17. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI

NOTE:

1. HVAC TO BE IN ATTIC. VERIFY WITH BUILDER.

2. UPPER FLOOR CEILING HEIGHTS TO BE 8'0" UNLESS NOTED.

3. UPPER FLOOR JOISTS TO BE 16 1/2" OPEN WEB FLOOR.

TRUSSES AT 16"OC w/3/4 T&G ADVANTECH FLOOR GLUED AND SCREWED. SEE TRUSS MANUFACTURER FOR FLOOR TRUSS LAYOUTS. ALL OPEN WEB FLOOR TRUSSES TO BE DESIGNED AND ENGINEERED BY TRUSS MANUFACTURER. TRUSS MANUFACTURER WILL PROVIDE TRUSS LAYOUT BASED ON ENGINEERING.

TRUSS MANUFACTURER TO SUPPLY TRUSSES WITH CHAMFERED END ON SELECTED UNITS TO ALLOW FOR EXTERIOR WALL SUPPORT AND RATER CLEARANCE.

4. HVAC AND WATER HEATER TO BE IN ATTIC UNLESS OTHERWISE NOTED.

ELEVATION NOTES:

1. GUTTERS AND DOWNSPOUTS ARE NOT SHOWN FOR CLARITY. DOWNSPOUTS SHALL BE LOCATED TOWARDS THE FRONT AND REAR OF THE HOUSE. LOCATE DOWNSPOUTS IN NON-VISUALLY OFFENSIVE LOCATIONS. FOR EXAMPLE, FRONT WALL OF HOUSE BESIDE PORCH COLUMNS, ETC. GENERAL CONTRACTOR SHALL VERIFY EXISTING GRADES AND COORDINATE ANY NECESSARY ADJUSTMENTS TO HOUSE WITH OWNER.

2. PLUMBING AND HVAC VENTS SHALL BE GROPED IN ATTIC TO LIMIT ROOF PENETRATIONS AND TO BE LOCATED AWAY FROM PUBLIC VIEW. I.E. AT THE REAR OF THE HOUSE AND SHALL BE PRIMED AND PAINTED TO MATCH ROOF COLOR.

3. PROVIDE ATTIC VENTILATION PER LOCAL CODE REQUIREMENTS.

4. EXTERIOR FLASHING SHALL BE CORRECTLY INSTALLED AT ALL CONNECTIONS BETWEEN ROOFS, WALLS, CHIMNEYS, PROJECTIONS, AND PENETRATIONS AS REQUIRED BY APPROVED CONSTRUCTION PRACTICES.

5. CONTRACTOR SHALL PROVIDE ADEQUATE ATTIC VENTILATIONS ROOF VENTS PER LOCAL GOVERNING CODE. INSTALL CONTINUOUS RIDGE VENTILATION AND PAINT TO MATCH ROOF. PROVIDE APPROPRIATE SOFFIT VENTILATION AT OVERHANGS.

FRAMING NOTES:

1. RAFTERS TO BE SUPPORTED BY CONTINUOUS BRACING FOR HORIZONTAL SPANS OF 15'0" OR GREATER.

2. SUPPORT ALL HIP, VALLEY, AND RIDGES @ 8'0" OC MAX.

3. ALL RAFTERS TO BEAR ON SECOND FLOOR WALLS WHERE APPLICABLE.

4. RAFTERS MAY BE SPLICED ONLY @ CONT. BRACING OR SECOND FLOOR WALLS.

5. RAFTERS TO BE PLACED IN COMPLIANCE WITH ALL LOCAL CODES. EXAMPLES:

2X6 RAFTER@16"OC MAX WITH 1/2" P W DECKING

2X6 RAFTERS @ 24"OC MAX WITH 5/8"P W DECKING

2X8 RAFTERS @ 24"OC MAX WITH 5/8"P W DECKING

2X8 RAFTERS @ 16"OC MAX WITH 1/2" P W DECKING

6. FASCIA OVERHANG TO BE 12" (TYPICAL) UNLESS NOTED ON ELEVATIONS.

7. ALL HIP/VALLEY RAFTERS TO BE 2X10 UNLESS NOTED.

NOTE:

PURLINS ARE PERMITTED TO BE INSTALLED TO REDUCE THE SPAN OF RAFTERS. PURLINS SHALL BE SUPPORTED BY 2 INCH X 4 INCH BRACES INSTALLED TO BEARING WALLS AT A SLOPE OF NOT LESS THAN 45 DEGREES. THE BRACES SHALL NOT BE SPACED MORE THAN 48" APART ON CENTER AND THE UNBRACED LENGTH OF BRACES SHALL NOT EXCEED 8 FT. PURLINS SHALL BE CONTINUOUS (REFER IRC R802.5.1)

FLOOR PLANS NOTES:

1. ALL STRUCTURAL INFORMATION SHOWN FOR REFERENCE PURPOSES ONLY. CONTRACTOR SHALL HAVE LICENSED STRUCTURAL ENGINEER REVIEW AND DESIGN ALL STRUCTURAL ELEMENTS SUCH AS ALL FRAMING WALLS, BEAMS, CONNECTIONS, HEADERS, JOISTS, AND RAFTERS.

2. ALL DIMENSIONS ARE FROM FACE OF STUD TO FACE OF STUD UNLESS NOTED OTHERWISE.

3. WINDOW SIZES INDICATED ON PLANS ARE NOTED BY APPROXIMATE ROUGH OPENING SIZE; REFER TO PLANS AND EXTERIOR ELEVATIONS FOR WINDOW TYPES.

4. COORDINATE LOCATION OF UTILITY METERS WITH SITE PLAN AND LOCATE AWAY FROM PUBLIC VIEW. VISUAL IMPACT SHALL BE MINIMIZED, I.E. MOUNT AS LOW AS POSSIBLE.

5. PREFABRICATED FIREPLACE CONSTRUCTION SHALL MEET OR EXCEED ALL APPLICABLE CODES REGARDING USE OF FIRE SEPARATIONS, CLEARANCES, ETC. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT ALL ITEMS AND CONSTRUCTION MEET OR EXCEED CODE. OVERALL FLUE HEIGHT SHALL BE COORDINATED TO MATCH HEIGHT SHOWN ON PLANS AND SHALL NOT EXCEED THE TOP OF CHIMNEY CHASES AS CONSTRUCTED.

6. CONTRACTOR SHALL COORDINATE ALL CLOSET SHELIVING REQUIREMENTS.

7. DO NOT SCALE DRAWINGS. FOLLOW DIMENSIONS.

8. CONTRACTOR SHALL FIELD VERIFY ALL CABINET DIMENSIONS BEFORE FABRICATION.

9. BEDROOM WINDOWS SHALL HAVE A MINIMUM NET CLEAR OPENING OF 5.7 SQ FT. A MINIMUM NET CLEAR OPENABLE WIDTH OF 20", A MINIMUM NET CLEAR OPENABLE HEIGHT OF 24" AND HAVE A MAXIMUM FINISH SILL HEIGHT OF 43" FROM FINISH FLOOR.

10. ALL GLASS LOCATED WITHIN 18" OF FLOOR, 12" OF A DOOR OR LOCATED WITHIN 60" OF FLOOR AT BATHTUBS, WHIRLPOOLS, SHOWERS, SAUNAS, STEAM ROOMS, OR HOT TUBS SHALL BE TEMPERED.

11. ALL EXPOSED INSULATION SHALL HAVE A FLAME SPREAD RATING OF LESS THAN 25 AND A SMOKE DENSITY RATING OF LESS THAN 450.

12. PROVIDE COMBUSTION AIR VENTS, WITH SCREEN AND BACK DAMPER, FOR FIREPLACES, WOOD STOVES AND ANY APPLIANCE WITH AN OPEN FLAME.

13. BATHROOMS AND UTILITY ROOMS SHALL BE VENTED TO THE OUTSIDE WITH A MINIMUM OF A 90 CFM FAN. RANGE HOODS SHALL ALSO BE VENTED TO OUTSIDE.

14. ATTIC HVAC UNITS SHALL BE LOCATED WITHIN 20' OF ITS SERVICE OPENING. RETURN AIR GRILLES SHALL NOT BE LOCATED WITHIN 10 FEET OF A GAS FIRED APPLIANCE.

15. ALL WALLS AND CEILINGS IN GARAGE AND GARAGE STORAGE AREAS TO HAVE 5/8 TYPE X GYPSUM BOARD WITH 1 HOUR FIRE RATING. ALL EXTERIOR DOORS IN GARAGE TO BE METAL OR SOLID CORE DOORS, INCLUDING DOORS ENTERING HEAT/COOLED PORTION OF RESIDENCE.

16. ALL FIREPLACE CHASE WALLS SHALL BE INSULATED INSIDE AND OUTSIDE. PROVIDE HORIZONTAL "DRAFT STOPS" AT EACH FLOOR LEVEL BY PACKING 6"(R-19) INSULATION BETWEEN 2X4 JOISTS.

17. ALL INTERIOR WALLS SHALL BE COVERED WITH 1/2" GYPSUM BOARD WITH METAL CORNER REINFORCING, TAPE FLOAT, AND SAND (3 COATS). USE 5/8" GYPSUM BOARD ON CEILINGS WHEN SUPPORTING MEMBERS ARE 24"OC OR GREATER. USE 1/2" GYPSUM BOARD ON CEILINGS WHEN SUPPORTING MEMBERS LESS THAN 24"OC.

18. ALL BATH AND TOILET AREA WALLS AND CEILINGS SHALL HAVE WATER RESISTANT GYPSUM BOARD.

REVISION TABLE	NUMBER	DATE	REVISED BY	DESCRIPTION

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DATE:

8/8/2022

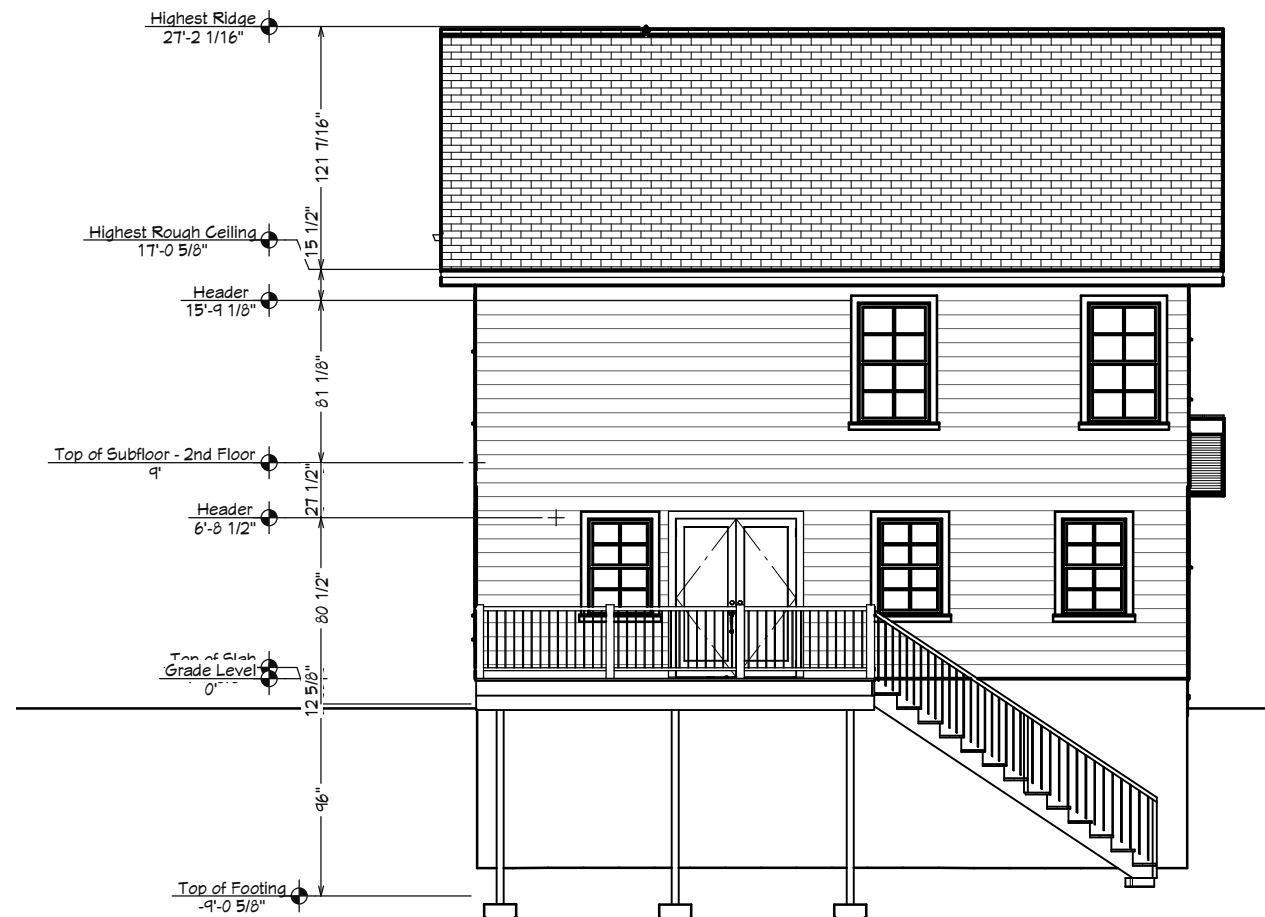
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Proposed Front Elevation
Scale: 1/8" = 1'



Proposed Rear Elevation
Scale: 1/8" = 1'

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DATE:

8/8/2022

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Proposed Left Elevation
Scale: 1/8" = 1'



Proposed Right Elevation
Scale: 1/8" = 1'

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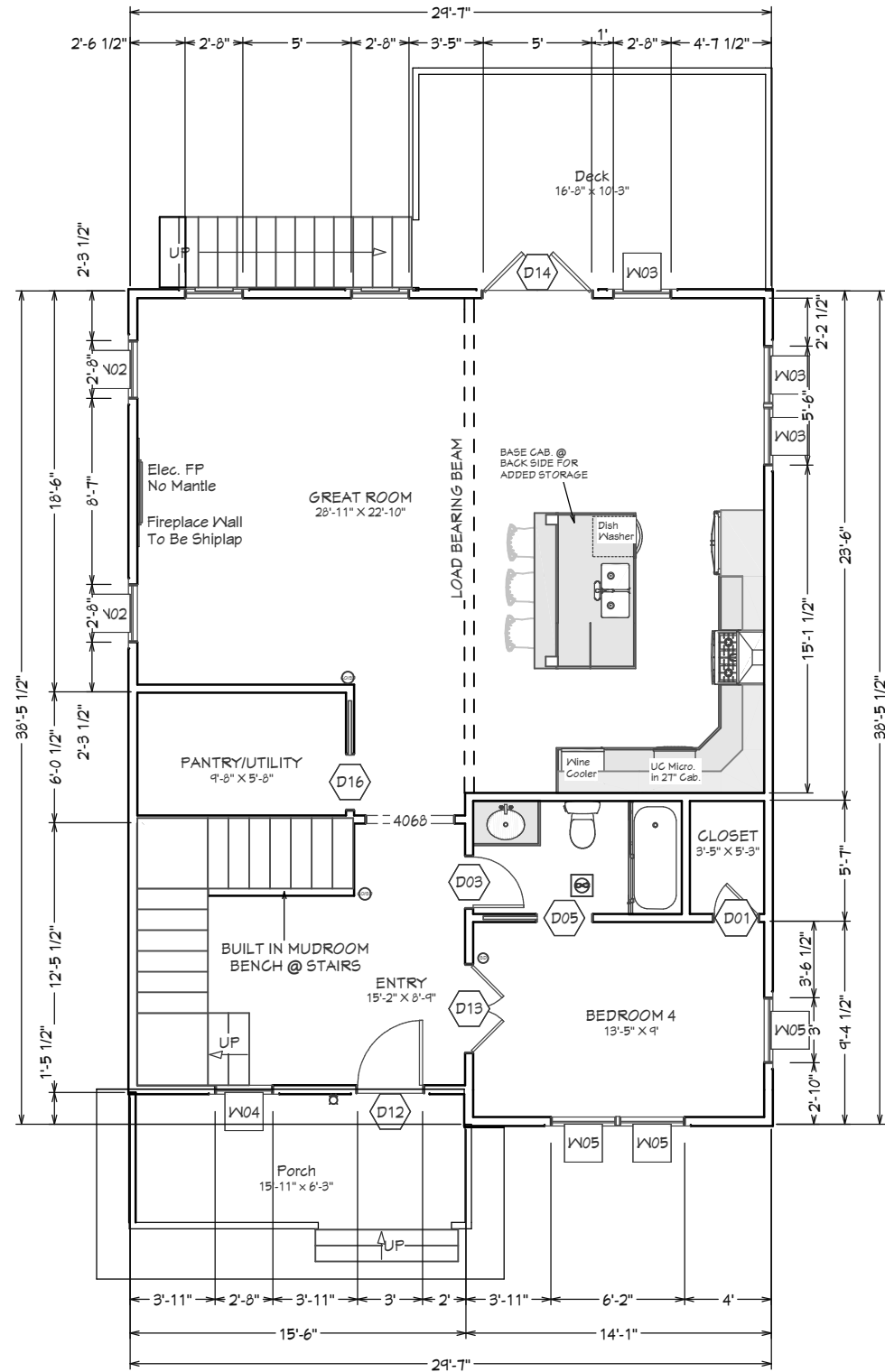
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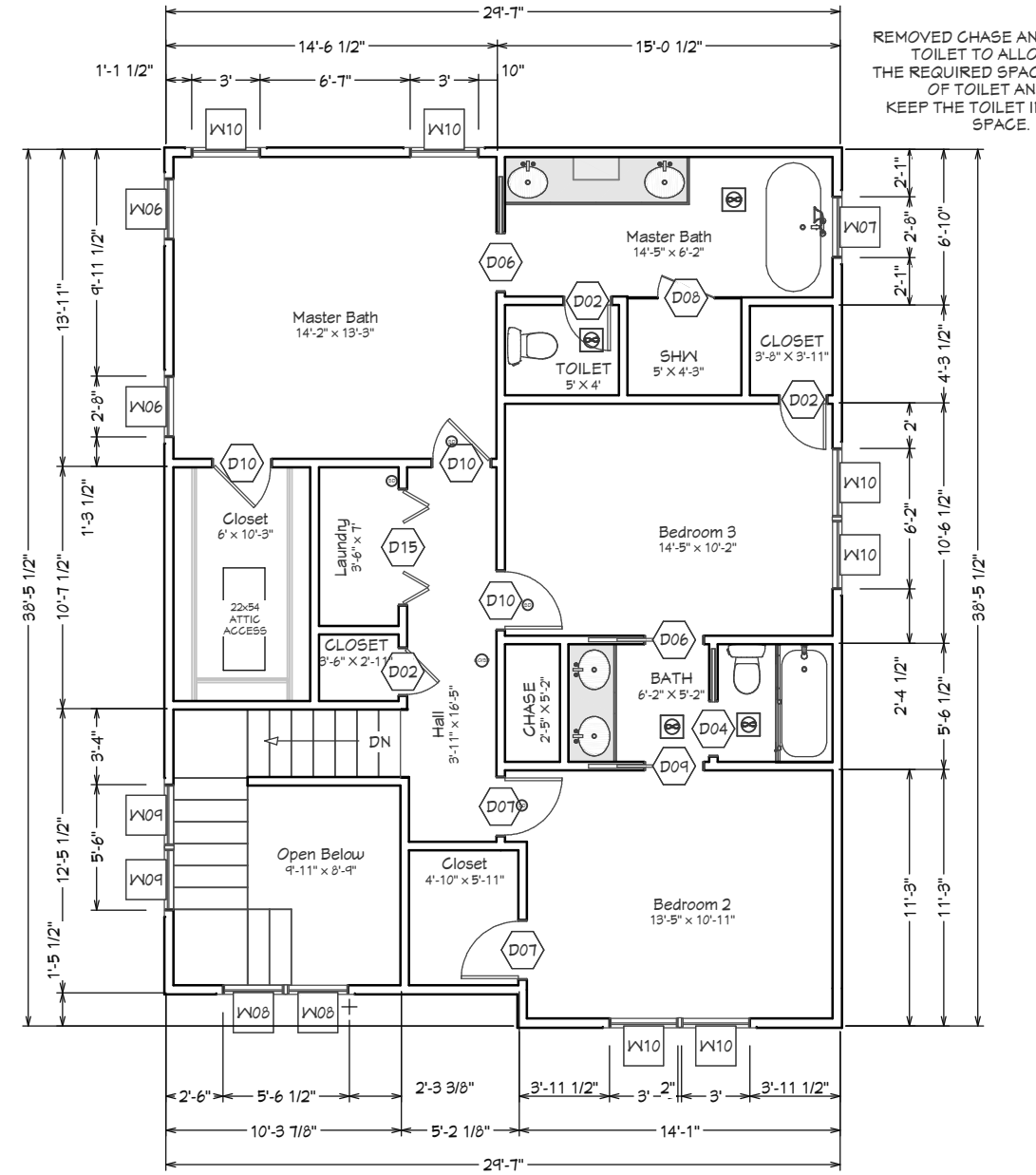
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8' CEILING HEIGHT



Proposed First Floor
Scale: 1/8" = 1'



Proposed Second Floor
Scale: 1/8" = 1'

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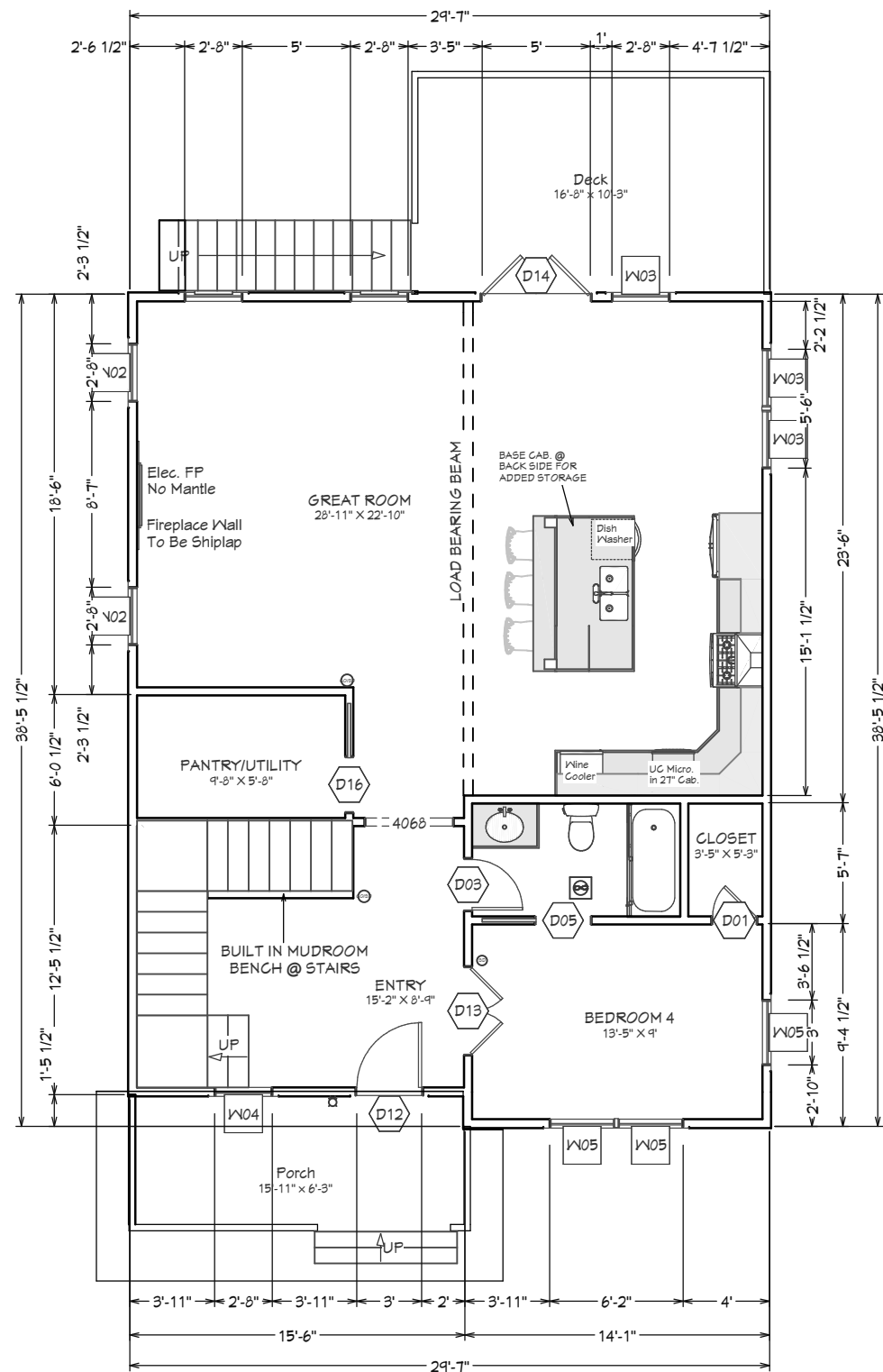
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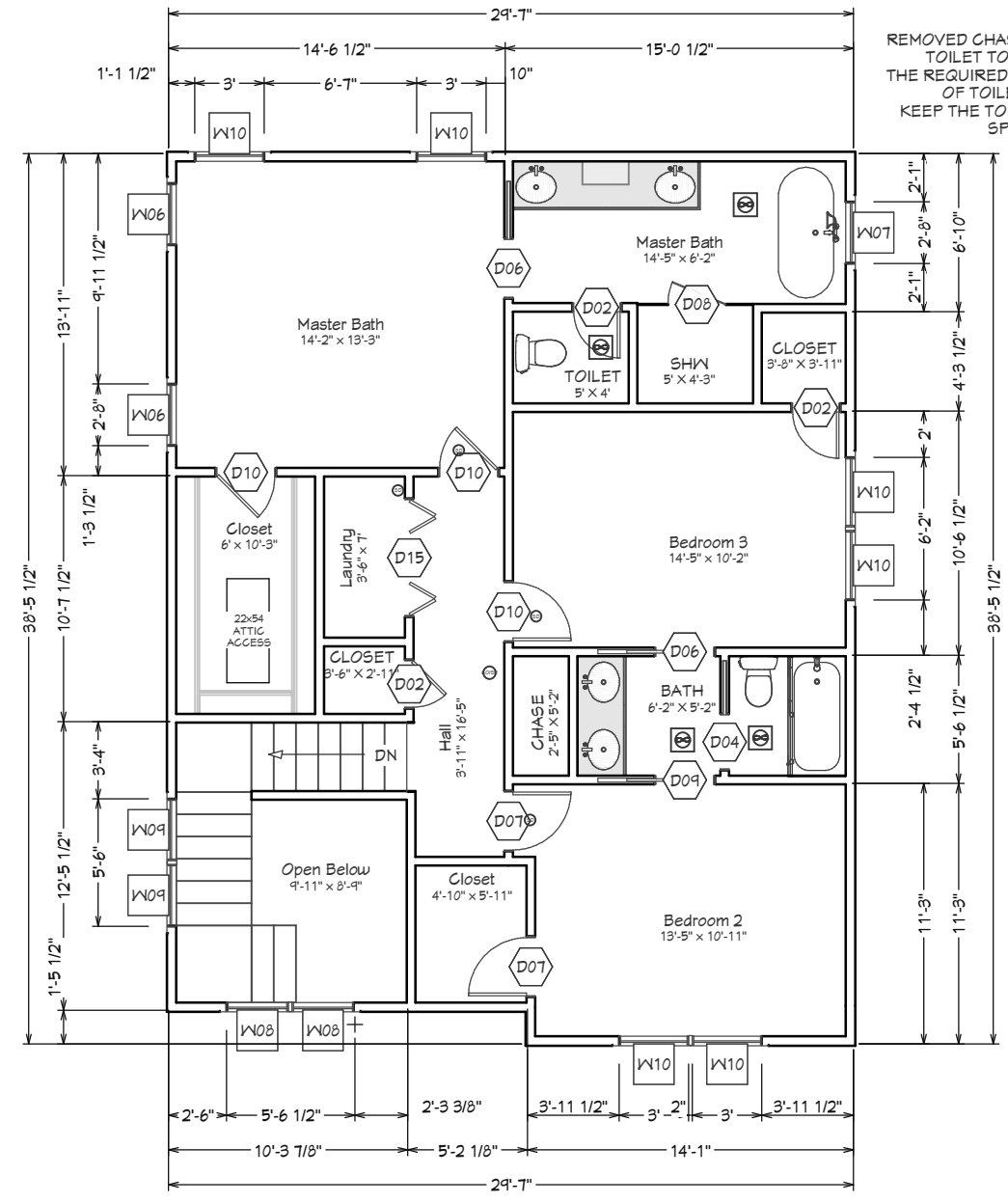
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Proposed First Floor
Scale: 1/8" = 1'



Proposed Second Floor
Scale: 1/8" = 1'

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DATE:

8/8/2022

SCALE:

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DOOR SCHEDULE

NUMBER	LABEL	QTY	FLOOR	SIZE	WIDTH	HEIGHT	R/O	DESCRIPTION	HEADER	THICKNESS	FIRE
D01	2068	1	1	2068 R IN	24 "	80 "	26"X82 1/2"	HINGED-PANEL	2X6X29" (2)	1 3/8"	
D02	2068	3	2	2068 L IN	24 "	80 "	26"X82 1/2"	HINGED-PANEL	2X6X29" (2)	1 3/8"	
D03	2468	1	1	2468 R IN	28 "	80 "	30"X82 1/2"	HINGED-PANEL	2X6X33" (2)	1 3/8"	
D04	2468	1	2	2468 R	28 "	80 "	58"X82 1/2"	POCKET-PANEL	2X6X61" (2)	1 3/8"	
D05	2668	1	1	2668 L	30 "	80 "	62"X82 1/2"	POCKET-PANEL	2X6X65" (2)	1 3/8"	
D06	2668	2	2	2668 L	30 "	80 "	62"X82 1/2"	POCKET-PANEL	2X6X65" (2)	1 3/8"	
D07	2668	2	2	2668 L IN	30 "	80 "	32"X82 1/2"	HINGED-PANEL	2X6X35" (2)	1 3/8"	
D08	2668	1	2	2668 R	30 "	80 "	30"X80"	SHOWER-GLASS SLAB		1/2"	
D09	2668	1	2	2668 R	30 "	80 "	62"X82 1/2"	POCKET-PANEL	2X6X65" (2)	1 3/8"	
D10	2668	3	2	2668 R IN	30 "	80 "	32"X82 1/2"	HINGED-PANEL	2X6X35" (2)	1 3/8"	
D11	3053	1	0	3053 L EX	36 "	62 11/16 "	38"X65 11/16"	EXT. HINGED-SLAB	2X6X41" (2)	1 3/4"	
D12	3068	1	1	3068 R EX	36 "	80 "	38"X83"	EXT. HINGED-DOOR E21	2X6X41" (2)	1 3/4"	
D13	4068	1	1	4068 L/R IN	48 "	80 "	50"X82 1/2"	DOUBLE HINGED-PANEL	2X8X53" (2)	1 3/8"	
D14	5068	1	1	5068 L/R EX	60 "	80 "	62"X83"	EXT. DOUBLE HINGED-DOOR F01	2X8X65" (2)	1 3/4"	
D15	5068	1	2	5068 L/R	60 "	80 "	62"X82 1/2"	4 DR. BIFOLD-PANEL	2X8X65" (2)	1 3/8"	
D16	2668	1	1	2668 R	30 "	80 "	62"X82 1/2"	POCKET-PANEL	2X6X65" (2)	1 3/8"	

WINDOW SCHEDULE

NUMBER	LABEL	QTY	FLOOR	SIZE	WIDTH	HEIGHT	R/O	EGRESS	DESCRIPTION	HEADER	TEMPERED
W01	1220	3	3	1220	14 "	24 "	15"X25"		LOUVERED	2X6X18" (2)	
W02	W1	2	1	2820FX	32 "	24 "	33"X25"		FIXED GLASS	2X6X36" (2)	
W03	W1	5	1	2840DH	32 "	48 "	33"X49"		DOUBLE HUNG	2X6X36" (2)	
W04	W1	1	1	2840DH	32 "	48 "	33"X49"	YES	DOUBLE HUNG	2X6X36" (2)	
W05	W1	3	1	3050DH	36 "	60 "	37"X61"	YES	DOUBLE HUNG	2X6X40" (2)	
W06	W1	2	2	2816FX	32 "	18 "	33"X19"		FIXED GLASS	2X6X36" (2)	
W07	W1	1	2	2820FX	32 "	24 "	33"X25"		FIXED GLASS	2X6X36" (2)	YES
W08	W1	2	2	2840DH	32 "	48 "	33"X49"		DOUBLE HUNG	2X6X36" (2)	
W09	W1	2	2	2840DH	32 "	48 "	33"X49"	YES	DOUBLE HUNG	2X6X36" (2)	
W10	W1	6	2	3050DH	36 "	60 "	37"X61"	YES	DOUBLE HUNG	2X6X40" (2)	

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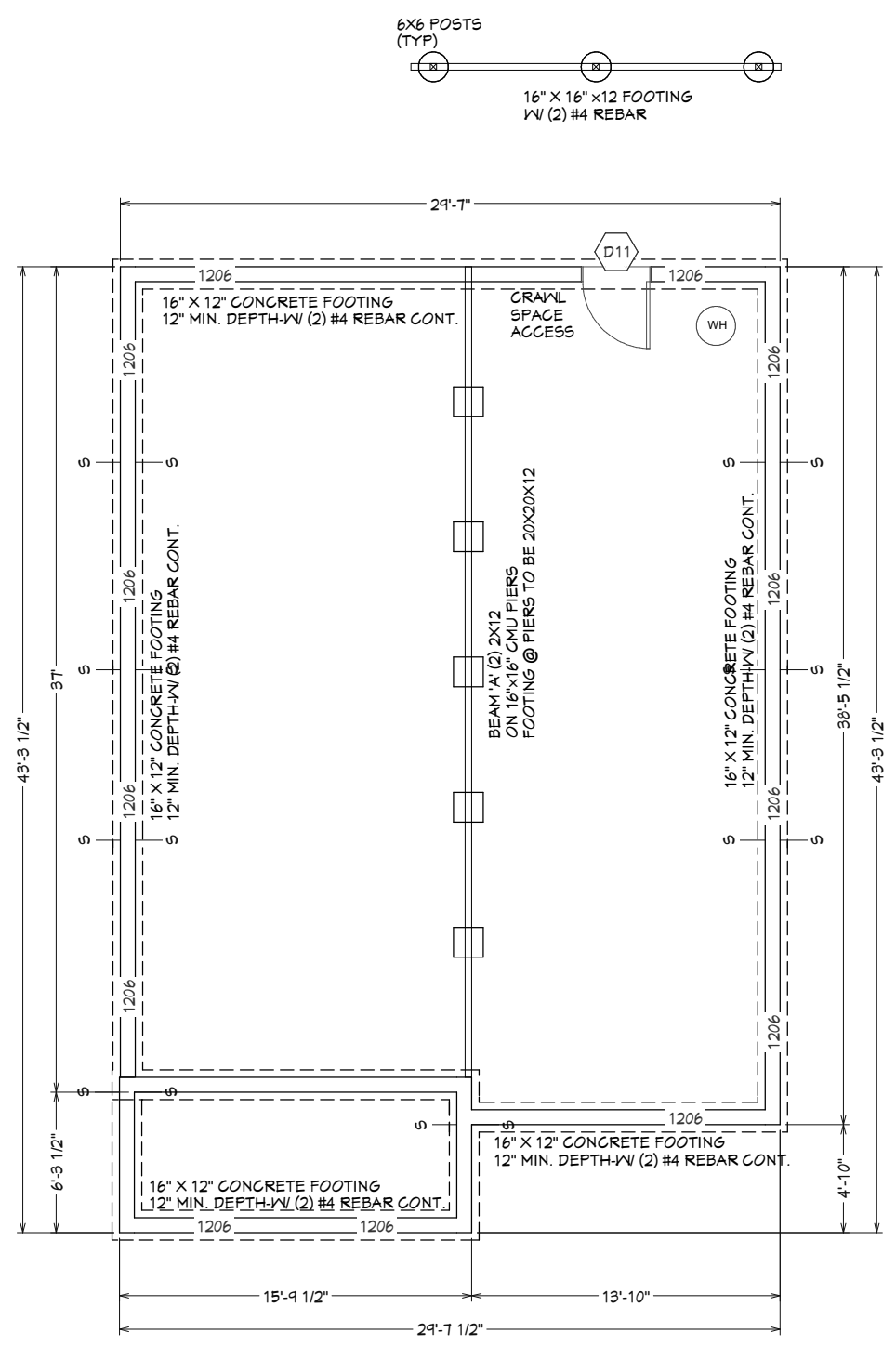
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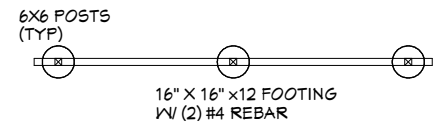
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Foundation Plan
Scale: 1/8" = 1'

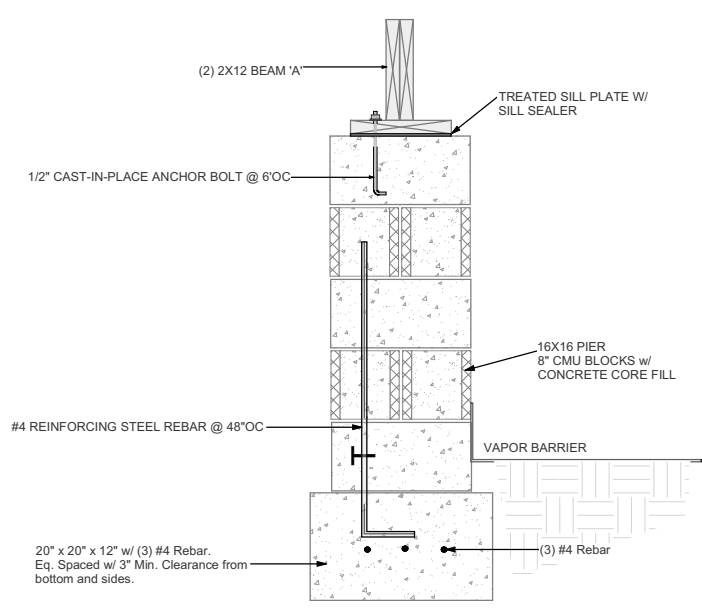


Required Beam Depths and Bearing Lengths [in] VERSA-LAM 2.0 3100

Load Duration %	Floor Load [psf]		Beam Support Spacing [Feet]	Width of Building Segment [feet]																	
	Live	Dead		KEY: Beam Breadth [in] X Beam Depth [in]					End Support / Intermediate Support Bearing Length Requirements [in]												
				20	24	26	28	30	32	36	40										
100%	40	10	8	3.5 x 7.25	1.5/3	3.5 x 7.25	1.5/3	3.5 x 9.5	1.5/3	3.5 x 9.5	1.5/3	3.5 x 9.5	1.5/4.5	3.5 x 9.5	1.5/4.5	3.5 x 9.5	3/4.5	3.5 x 9.5	3/4.5		
				5.25 x 7.25	1.5/1.5	5.25 x 7.25	1.5/3	5.25 x 7.25	1.5/3	5.25 x 7.25	1.5/3	5.25 x 7.25	1.5/3	5.25 x 7.25	1.5/3	5.25 x 7.25	1.5/3	5.25 x 9.5	1.5/3	5.25 x 9.5	1.5/3
			10	3.5 x 9.5	1.5/3	3.5 x 9.5	1.5/4.5	3.5 x 9.5	1.5/4.5	3.5 x 9.5	1.5/4.5	3.5 x 11.875	3/4.5	3.5 x 11.875	3/4.5	3.5 x 11.875	3/4.5	3.5 x 11.875	3/6	3.5 x 11.875	3/6
				5.25 x 9.5	1.5/3	5.25 x 9.5	1.5/3	5.25 x 9.5	1.5/3	5.25 x 9.5	1.5/3	5.25 x 9.5	1.5/3	5.25 x 9.5	1.5/3	5.25 x 9.5	1.5/4.5	5.25 x 9.5	1.5/4.5	5.25 x 9.5	1.5/4.5
			12	3.5 x 11.875	1.5/4.5	3.5 x 11.875	3/4.5	3.5 x 11.875	3/4.5	3.5 x 11.875	3/4.5	3.5 x 11.875	3/4.5	3.5 x 11.875	3/6	3.5 x 11.875	3/6	3.5 x 14	3/6	3.5 x 14	3/7.5
				5.25 x 9.5	1.5/3	5.25 x 9.5	1.5/3	5.25 x 11.875	1.5/3	5.25 x 11.875	1.5/3	5.25 x 11.875	1.5/3	5.25 x 11.875	1.5/4.5	5.25 x 11.875	1.5/4.5	5.25 x 11.875	3/4.5	5.25 x 11.875	3/4.5
			14	3.5 x 11.875	1.5/4.5	3.5 x 14	3/4.5	3.5 x 14	3/6	3.5 x 14	3/6	3.5 x 14	3/6	3.5 x 14	3/6	3.5 x 14	3/6	3.5 x 16	3/7.5	3.5 x 16	3/7.5
				5.25 x 11.875	1.5/3	5.25 x 11.875	1.5/3	5.25 x 11.875	1.5/4.5	5.25 x 11.875	1.5/4.5	5.25 x 11.875	1.5/4.5	5.25 x 11.875	1.5/4.5	5.25 x 14	3/4.5	5.25 x 14	3/4.5	5.25 x 14	3/6
			16	3.5 x 14	3/4.5	3.5 x 16	3/6	3.5 x 16	3/6	3.5 x 16	3/6	3.5 x 16	3/6	3.5 x 16	3/7.5	3.5 x 16	3/7.5	3.5 x 18	4.5/9	3.5 x 18	4.5/9
				5.25 x 11.875	1.5/3	5.25 x 14	1.5/4.5	5.25 x 14	1.5/4.5	5.25 x 14	1.5/4.5	5.25 x 14	1.5/4.5	5.25 x 14	3/4.5	5.25 x 14	3/4.5	5.25 x 16	3/6	5.25 x 16	3/6
			18	3.5 x 16	3/6	3.5 x 16	3/6	3.5 x 18	3/7.5	3.5 x 18	3/7.5	3.5 x 18	3/7.5	3.5 x 18	4.5/9	3.5 x 18	4.5/9	5.25 x 16	3/6	5.25 x 18	3/7.5
				5.25 x 14	1.5/4.5	5.25 x 14	3/4.5	5.25 x 16	3/4.5	5.25 x 16	3/4.5	5.25 x 16	3/6	5.25 x 16	3/6	7 x 16	3/4.5	7 x 16	3/6	7 x 16	3/6
			20	3.5 x 18	3/6	3.5 x 18	3/7.5	5.25 x 16	3/6	5.25 x 18	3/6	5.25 x 18	3/6	5.25 x 18	3/6	5.25 x 18	3/6	-	-	-	-
				5.25 x 16	1.5/4.5	5.25 x 16	3/4.5	7 x 16	1.5/4.5	7 x 16	1.5/4.5	7 x 16	1.5/4.5	7 x 16	3/4.5	7 x 16	3/4.5	7 x 18	3/6	7 x 18	3/6

TABLE R403.1(1)
MINIMUM WIDTH AND THICKNESS FOR CONCRETE FOOTINGS FOR LIGHT-FRAME CONSTRUCTION (inches) ^{a, b}

SNOW LOAD OR ROOF LIVE LOAD	STORY AND TYPE OF STRUCTURE WITH LIGHT FRAME	LOAD-BEARING VALUE OF SOIL (psf)					
		1500	2000	2500	3000	3500	4000
20 psf	1 story—slab-on-grade	12 x 6	12 x 6	12 x 6	12 x 6	12 x 6	12 x 6
	1 story—with crawl space	12 x 6	12 x 6	12 x 6	12 x 6	12 x 6	12 x 6
	1 story—plus basement	18 x 6	14 x 6	12 x 6	12 x 6	12 x 6	12 x 6
	2 story—slab-on-grade	12 x 6	12 x 6	12 x 6	12 x 6	12 x 6	12 x 6
	2 story—with crawl space	16 x 6	12 x 6	12 x 6	12 x 6	12 x 6	12 x 6
	2 story—plus basement	22 x 6	16 x 6	13 x 6	12 x 6	12 x 6	12 x 6
	3 story—slab-on-grade	14 x 6	12 x 6	12 x 6	12 x 6	12 x 6	12 x 6
	3 story—with crawl space	19 x 6	14 x 6	12 x 6	12 x 6	12 x 6	12 x 6
	3 story—plus basement	25 x 8	19 x 6	15 x 6	13 x 6	12 x 6	12 x 6



INTERIOR SUPPORT DETAIL
SCALE: NTS

REVISION TABLE	REVISION BY	DESCRIPTION
NUMBER	DATE	

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NEW CONSTRUCTION
641 WARREN AVE.
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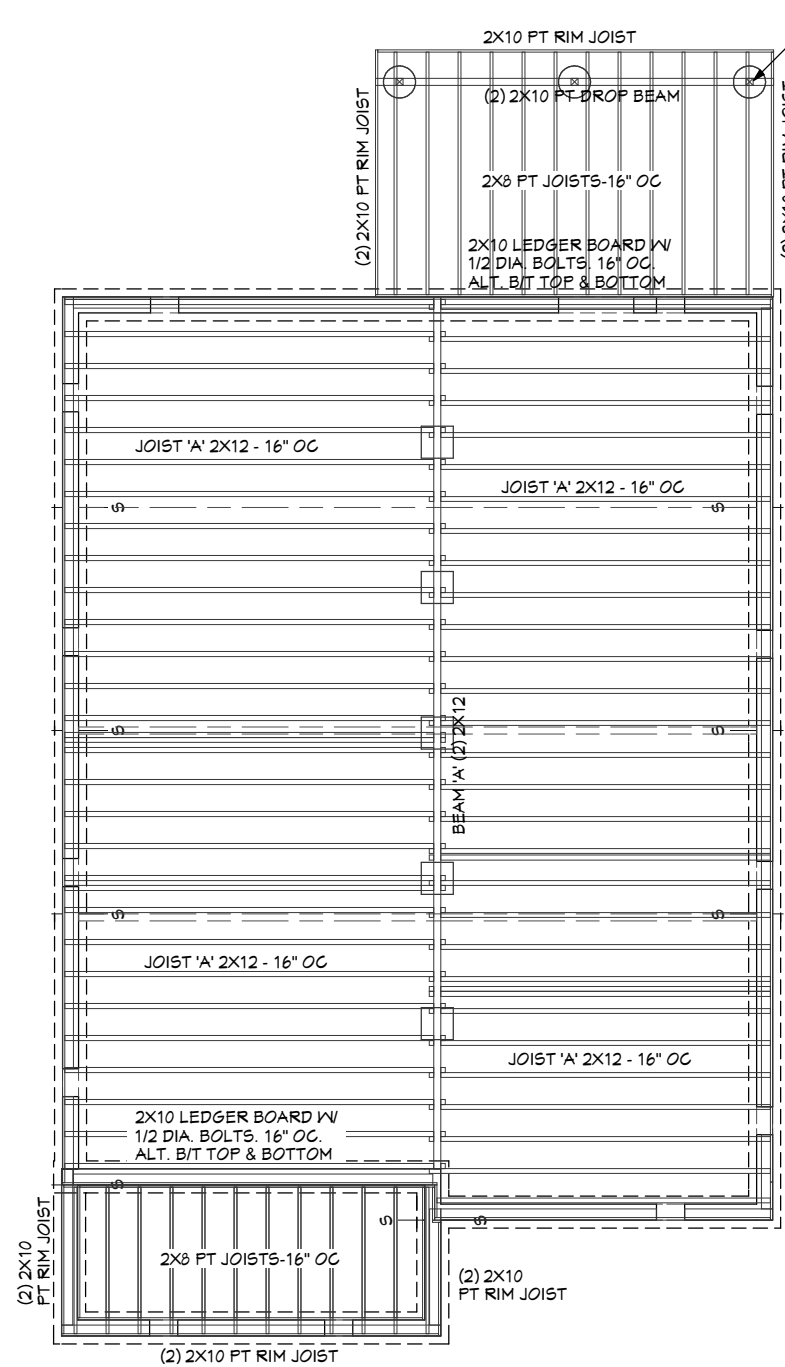
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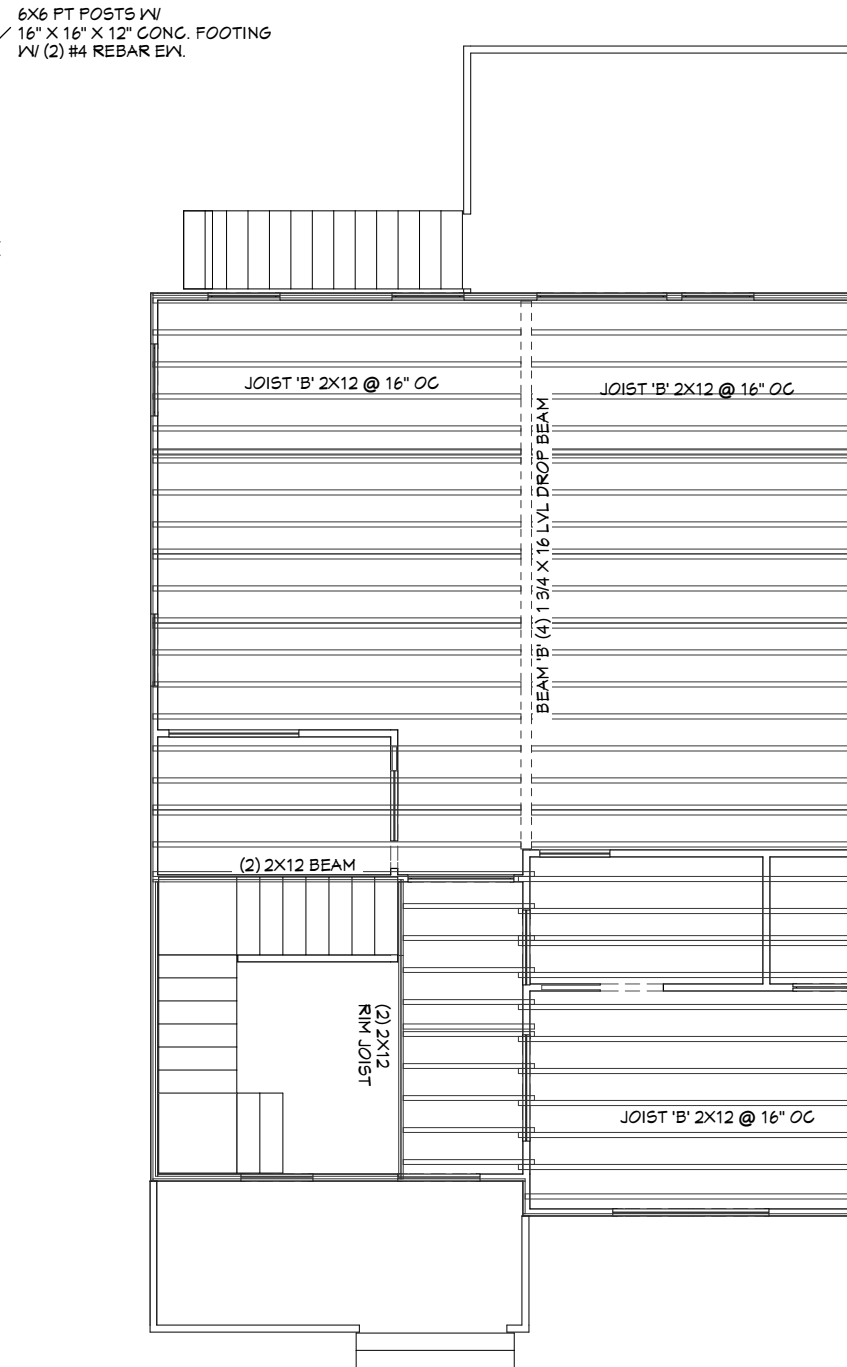
SCALE:

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S1.1



Floor Framing-First Floor
 Scale: 1/8" = 1'



Floor Framing-Second Floor
 Scale: 1/8" = 1'

TABLE R502.3.1(2)
 FLOOR JOIST SPANS FOR COMMON LUMBER SPECIES (Residential living areas, live load = 40 psf, L/Δ = 360) ^b

JOIST SPACING (inches)	SPECIES AND GRADE	DEAD LOAD = 10 psf				DEAD LOAD = 20 psf				
		2 x 6	2 x 8	2 x 10	2 x 12	2 x 6	2 x 8	2 x 10	2 x 12	
		Maximum floor joist spans (ft. - in.)								
16	Southern pine	SS	10-2	13-4	17-0	20-9	10-2	13-4	17-0	20-9
	Southern pine	#1	9-9	12-10	16-1	19-1	9-9	12-7	14-8	17-5
	Southern pine	#2	9-4	11-10	14-0	16-6	8-6	10-10	12-10	15-1
	Southern pine	#3	7-1	8-11	10-10	12-10	6-5	8-2	9-10	11-8
	Spruce-pine-fir	SS	9-6	12-7	16-0	19-6	9-6	12-7	16-0	19-6
	Spruce-pine-fir	#1	9-4	12-3	15-5	17-10	9-1	11-6	14-1	16-3
	Spruce-pine-fir	#2	9-4	12-3	15-5	17-10	9-1	11-6	14-1	16-3
	Spruce-pine-fir	#3	7-6	9-6	11-8	13-6	6-10	8-8	10-7	12-4

TABLE R502.3.1(1)
 FLOOR JOIST SPANS FOR COMMON LUMBER SPECIES (Residential sleeping areas, live load = 30 psf, L/Δ = 360) ^a

JOIST SPACING (inches)	SPECIES AND GRADE	DEAD LOAD = 10 psf				DEAD LOAD = 20 psf				
		2 x 6	2 x 8	2 x 10	2 x 12	2 x 6	2 x 8	2 x 10	2 x 12	
		Maximum floor joist spans (ft. - in.)								
12"	Southern pine	SS	12-3	16-2	20-8	25-1	12-3	16-2	20-8	25-1
	Southern pine	#1	11-10	15-7	19-10	24-2	11-10	15-7	18-7	22-0
	Southern pine	#2	11-3	14-11	18-1	21-4	10-9	13-8	16-2	19-1
	Southern pine	#3	9-2	11-6	14-0	16-6	8-2	10-3	12-6	14-9

REVISION TABLE	NUMBER	DATE	REVISED BY	DESCRIPTION

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NEW CONSTRUCTION
 641 WARREN AVE.
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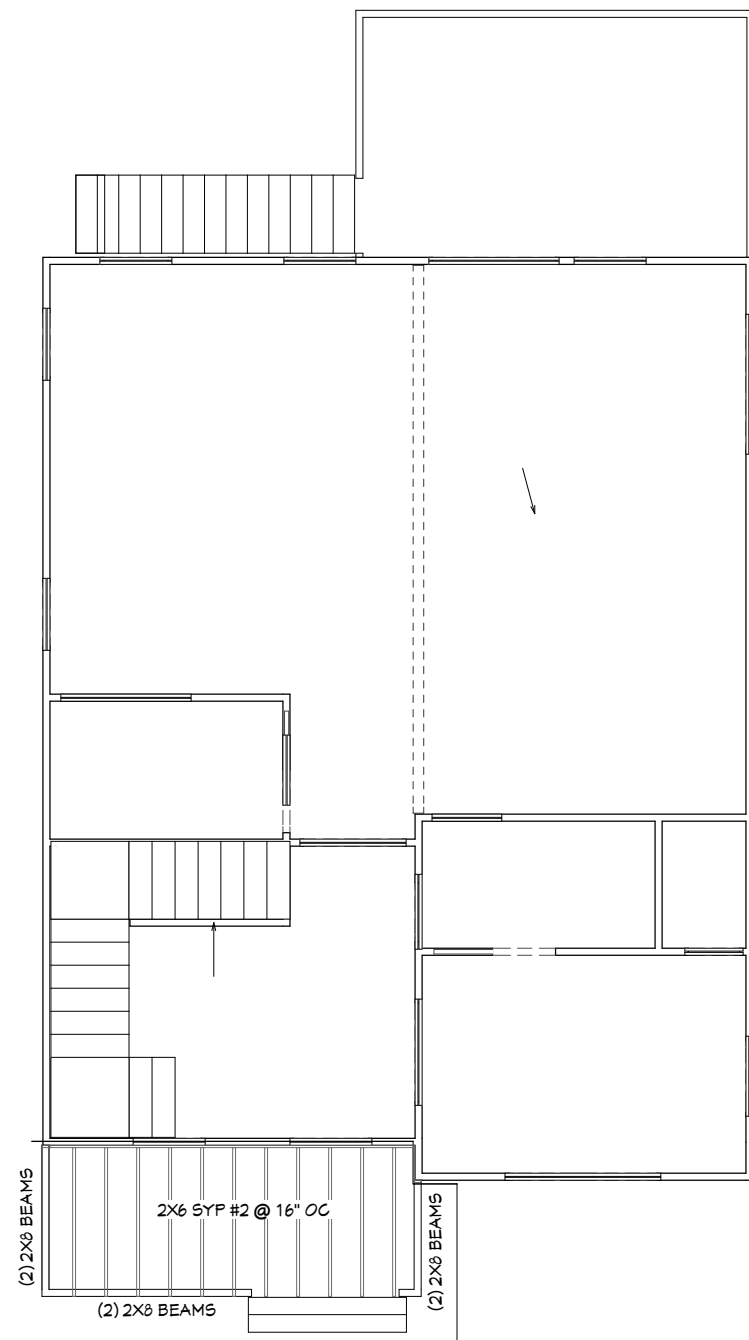
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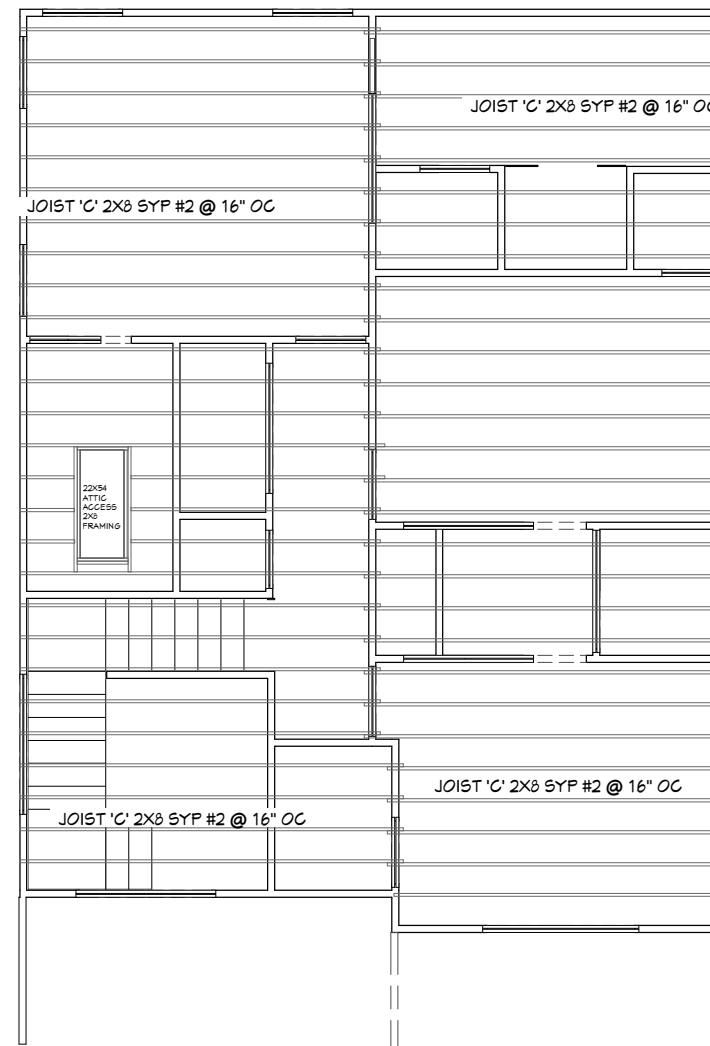
SCALE:

SHEET:

S1.2



First Floor-Ceiling Framing
Scale: 1/8" = 1'



Second Floor-Ceiling Framing
Scale: 1/8" = 1'

TABLE R602.7(3)
GIRDER AND HEADER SPANS^a FOR OPEN PORCHES (Maximum span for Douglas fir-larch, hem-fir, Southern pine and spruce-pine-fir^b)

SIZE	SUPPORTING ROOF						SUPPORTING FLOOR	
	GROUND SNOW LOAD (psf)							
	30		50		70			
	DEPTH OF PORCH ^c (feet)						8	14
	8	14	8	14	8	14		
2-2 x 6	7-6	5-8	6-2	4-8	5-4	4-0	6-4	4-9
2-2 x 8	10-1	7-7	8-3	6-2	7-1	5-4	8-5	6-4
2-2 x 10	12-4	9-4	10-1	7-7	8-9	6-7	10-4	7-9
2-2 x 12	14-4	10-10	11-8	8-10	10-1	7-8	11-11	9-0

TABLE R802.5.1(2)
CEILING JOIST SPANS FOR COMMON LUMBER SPECIES (Uninhabitable attics with limited storage, live load = 20 psf, L/Δ = 240)

CEILING JOIST SPACING (inches)	SPECIES AND GRADE	DEAD LOAD = 10 psf				
		2 x 4	2 x 6	2 x 8	2 x 10	
		Maximum ceiling joist spans				
		(feet - inches)				
16"	Southern pine	SS	9-4	14-7	19-3	24-7
	Southern pine	#1	8-11	14-0	17-9	20-9
	Southern pine	#2	8-0	12-0	15-3	18-1
	Southern pine	#3	6-2	9-2	11-6	14-0

REVISION TABLE	NUMBER	DATE	REVISED BY	DESCRIPTION

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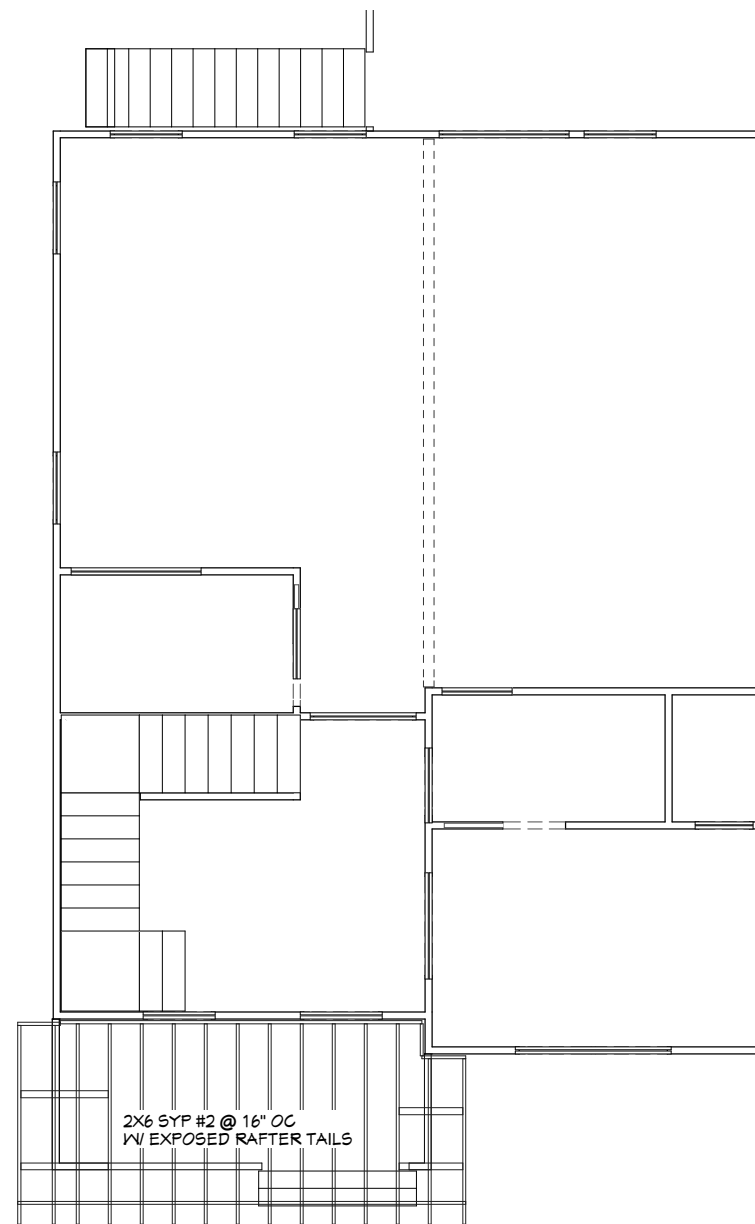
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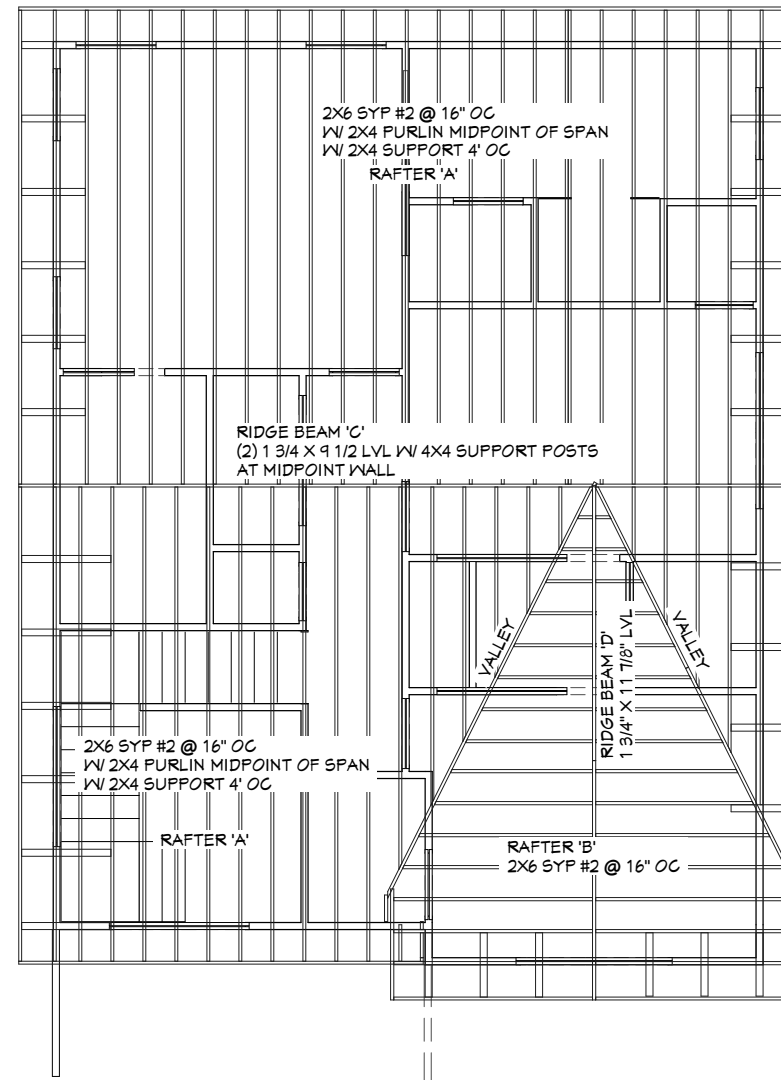
SCALE:

SHEET:

S1.3



First Floor-Roof Framing
Scale: 1/8" = 1'

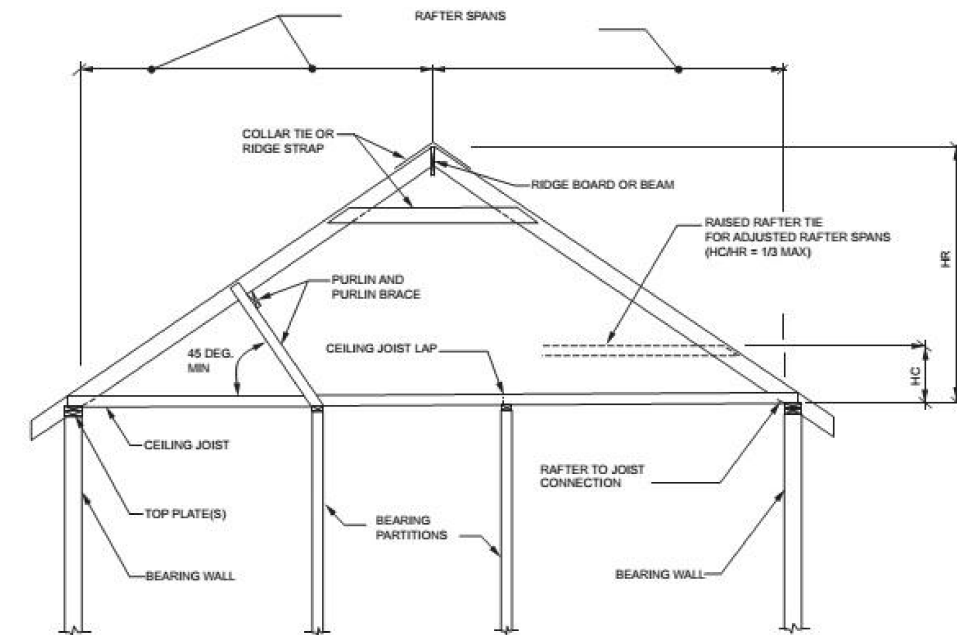


Second Floor-Roof Framing
Scale: 1/8" = 1'

TABLE R802.4.1(3)

RAFTER SPANS FOR COMMON LUMBER SPECIES (Ground snow load = 30 psf, ceiling not attached to rafters, L/Δ = 180)

RAFTER SPACING (inches)	SPECIES AND GRADE		DEAD LOAD = 10 psf					DEAD LOAD = 20 psf				
			2 x 4	2 x 6	2 x 8	2 x 10	2 x 12	2 x 4	2 x 6	2 x 8	2 x 10	2 x 12
			Maximum rafter spans ^a									
		(feet - inches)	(feet - inches)	(feet - inches)	(feet - inches)	(feet - inches)	(feet - inches)	(feet - inches)	(feet - inches)	(feet - inches)	(feet - inches)	(feet - inches)
16"	Southern pine	SS	8-11	14-1	18-6	23-8	Note b	8-11	14-1	18-5	1-11	25-11
	Southern pine	#1	8-7	13-0	16-6	19-3	22-10	7-10	11-7	14-9	17-3	20-5
	Southern pine	#2	7-6	11-2	14-2	16-10	19-10	6-8	10-0	12-8	15-1	17-9
	Southern pine	#3	5-9	8-6	10-8	13-0	15-4	5-2	7-7	9-7	11-7	13-9



For SI: 1 inch = 25.4 mm, 1 foot = 305 mm, 1 degree = 0.018 rad.

H_c = Height of ceiling joists or rafter ties measured vertically above the top of rafter support walls.

H_r = Height of roof ridge measured vertically above the top of the rafter support walls.

FIGURE R802.4.5
BRACED RAFTER CONSTRUCTION

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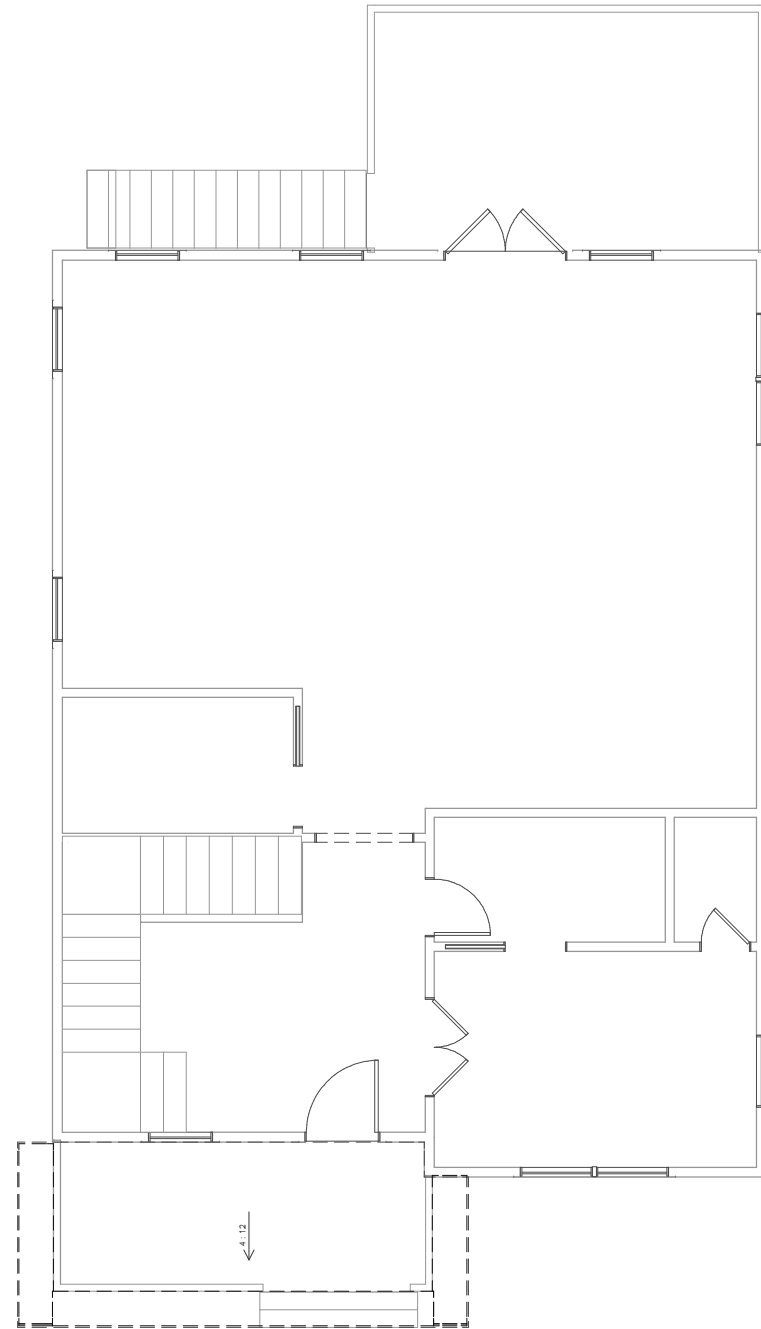
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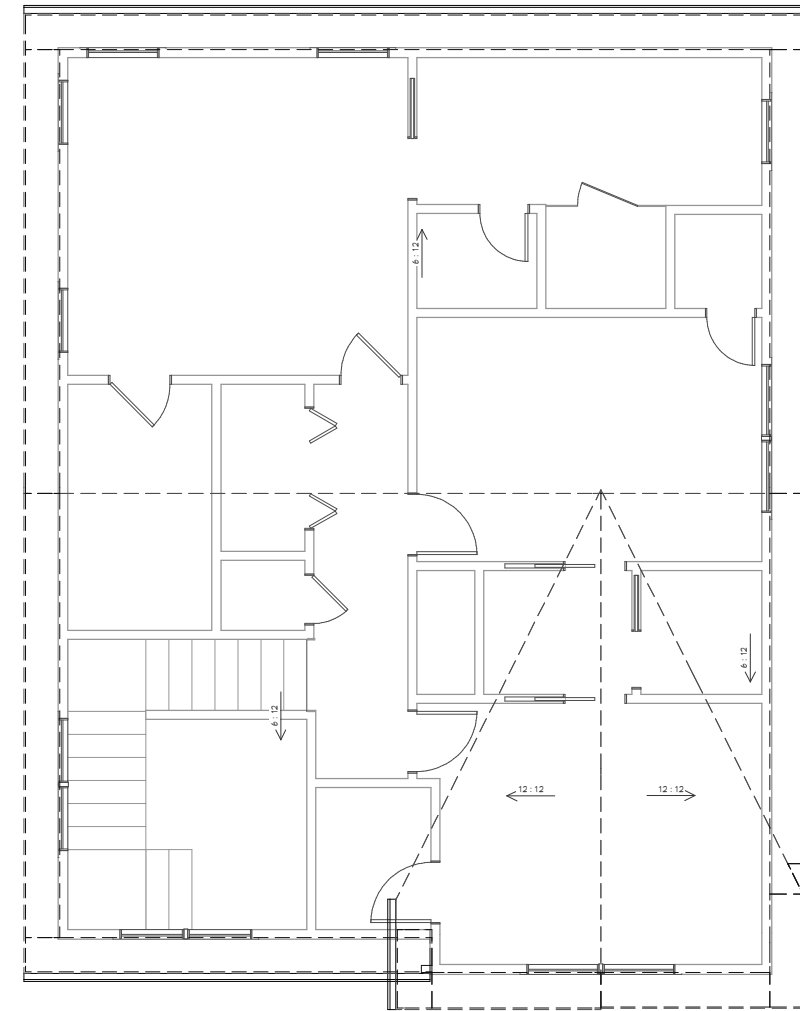
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First Floor-Roof Plan
Scale: 1/8" = 1'



Second Floor-Roof Plan
Scale: 1/8" = 1'

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641 WARREN AVE
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S1.5

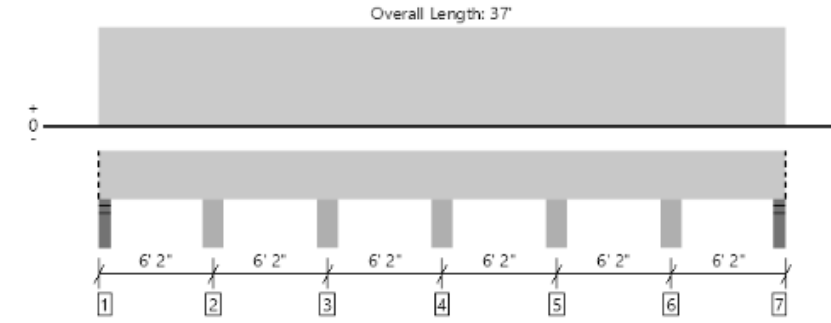
First Floor Framing Plan			
Member Name	Results	Current Solution	Comments
Beam "A"	Passed	2 piece(s) 2 x 12 SPF No.1/No.2	
Joist "A"	Passed	1 piece(s) 2 x 12 SPF No.1/No.2 @ 16" OC	
Second Floor Framing Plan			
Member Name	Results	Current Solution	Comments
Beam "B" (Drop Beam)	Passed	4 piece(s) 1 3/4" x 16" 2.0E Microllam® LVL	
Beam "B" (Steel Option Flush Beam)	Passed	1 piece(s) W12X40 (A992) ASTM Steel	
Joist "B"	Passed	1 piece(s) 2 x 12 SPF No.1/No.2 @ 16" OC	
Second Floor Ceiling Framing Plan			
Member Name	Results	Current Solution	Comments
Joist "C"	Passed	1 piece(s) 2 x 8 SPF No.1/No.2 @ 16" OC	
Second Floor Roof Framing Plan			
Member Name	Results	Current Solution	Comments
Ridge Beam "C"	Passed	2 piece(s) 1 3/4" x 9 1/2" 2.0E Microllam® LVL	
Ridge Beam "D"	Passed	1 piece(s) 1 3/4" x 11 7/8" 2.0E Microllam® LVL	
Rafter "A"	Passed	1 piece(s) 2 x 6 SPF No.1/No.2 @ 16" OC	
Rafter "B"	Passed	1 piece(s) 2 x 6 SPF No.1/No.2 @ 16" OC	

FORTEWEB Software Operator	Job Notes
C William Hamilton Inspection Wizards LLC (678) 770-4079 Bhamilton@InspectionWizards.com	



12/2/2021 4:03:58 AM UTC
ForteWEB v3.2
File Name: 641 Warren Avenue

First Floor Framing Plan, Beam "A"
2 piece(s) 2 x 12 SPF No.1/No.2



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	6564 @ 6' 2"	7650 (6.00")	Passed (86%)	--	1.0 D + 1.0 L (Adj Spans)
Shear (lbs)	2288 @ 32' 1/4"	3038	Passed (75%)	1.00	1.0 D + 1.0 L (Adj Spans)
Moment (Ft-lbs)	-3849 @ 30' 10"	4614	Passed (83%)	1.00	1.0 D + 1.0 L (Adj Spans)
Live Load Defl. (in)	0.027 @ 33' 11 5/8"	0.150	Passed (L/999+)	--	1.0 D + 1.0 L (Alt Spans)
Total Load Defl. (in)	0.035 @ 34' 3/8"	0.300	Passed (L/999+)	--	1.0 D + 1.0 L (Alt Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- Applicable calculations are based on NDS.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Total	
1 - Stud wall - SPF	3.50"	3.50"	1.96"	778	1716/-204	2494/-204	Blocking
2 - Column - SPF	6.00"	6.00"	5.15"	2122	4442	6564	None
3 - Column - SPF	6.00"	6.00"	4.81"	1842	4294	6136	None
4 - Column - SPF	6.00"	6.00"	4.96"	1933	4397	6330	None
5 - Column - SPF	6.00"	6.00"	4.81"	1842	4294	6136	None
6 - Column - SPF	6.00"	6.00"	5.15"	2122	4442	6564	None
7 - Stud wall - SPF	3.50"	3.50"	1.96"	778	1716/-204	2494/-204	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	14' o/c	
Bottom Edge (Lu)	9' 2" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Comments
0 - Self Weight (PLF)	0 to 37'	N/A	8.6	--	
1 - Uniform (PSF)	0 to 37' (Front)	15'	20.0	40.0	Default Load

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The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

FORTEWEB Software Operator	Job Notes
C William Hamilton Inspection Wizards LLC (678) 770-4079 Bhamilton@InspectionWizards.com	



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ForteWEB v3.2, Engine: V8.2.0.17, Data: V8.1.0.16
File Name: 641 Warren Avenue

REVISION TABLE
NUMBER DATE REVISION BY DESCRIPTION

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641 WARREN AVE.
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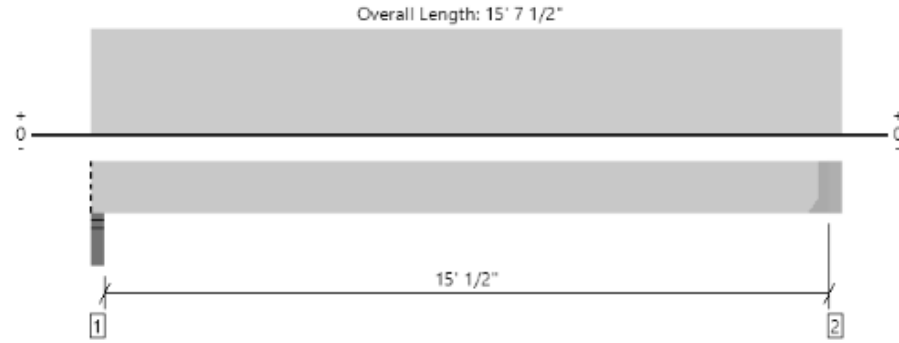
8/8/2022

SCALE:

SHEET:

S1.7

First Floor Framing Plan, Joist "A"
1 piece(s) 2 x 12 SPF No.1/No.2 @ 16" OC



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	605 @ 15' 4"	956 (1.50")	Passed (63%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	530 @ 14' 4 3/4"	1519	Passed (35%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	2288 @ 7' 9 1/4"	2653	Passed (86%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.252 @ 7' 9 1/4"	0.504	Passed (L/720)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.378 @ 7' 9 1/4"	0.756	Passed (L/480)	--	1.0 D + 1.0 L (All Spans)
TJ-Pro™ Rating	N/A	N/A	N/A	--	N/A

System : Floor
Member Type : Joist
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- A 15% increase in the moment capacity has been added to account for repetitive member usage.
- Applicable calculations are based on NDS.
- No composite action between deck and joist was considered in analysis.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Total	
1 - Stud wall - SPF	3.50"	3.50"	1.50"	207	414	621	Blocking
2 - Hanger on 11 1/4" LVL beam	3.50"	Hanger ¹	1.50"	209	419	628	See note ¹

- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.
- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- ¹ See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	4' 4" o/c	
Bottom Edge (Lu)	15' 4" o/c	

•Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie						
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
2 - Face Mount Hanger	LUS28	1.75"	N/A	6-10dx1.5	3-10d	

- Refer to manufacturer notes and instructions for proper installation and use of all connectors.

Vertical Load	Location (Side)	Spacing	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform (PSF)	0 to 15' 7 1/2"	16"	20.0	40.0	Default Load

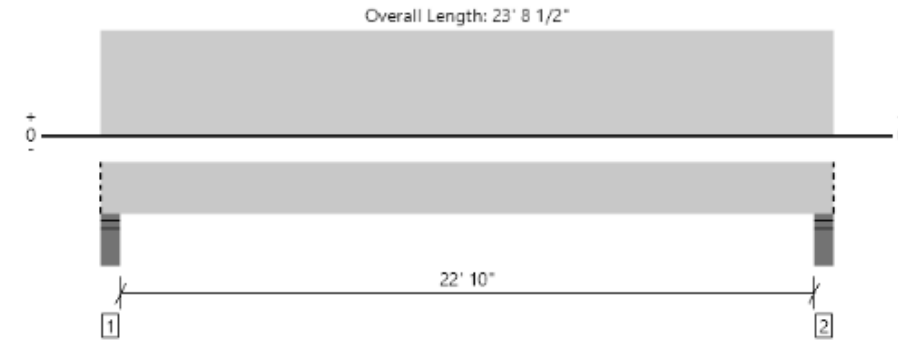
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File Name: 641 Warren Avenue
Page 3 / 11

Second Floor Framing Plan, Beam "B" (Drop Beam)
4 piece(s) 1 3/4" x 16" 2.OE Microllam® LVL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	9841 @ 3 3/4"	15619 (5.25")	Passed (63%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	8371 @ 1' 9 1/4"	21280	Passed (39%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	55293 @ 11' 10 1/4"	62228	Passed (89%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.713 @ 11' 10 1/4"	0.769	Passed (L/388)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	1.167 @ 11' 10 1/4"	1.154	Passed (L/237)	--	1.0 D + 1.0 L (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- Member should be side-loaded from both sides of the member or braced to prevent rotation.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Total	
1 - Stud wall - SPF	5.25"	5.25"	3.31"	3825	6016	9841	Blocking
2 - Stud wall - SPF	5.25"	5.25"	3.31"	3825	6016	9841	Blocking

- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	7' 4" o/c	
Bottom Edge (Lu)	23' 9" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Comments
0 - Self Weight (PLF)	0 to 23' 8 1/2"	N/A	32.7	--	
1 - Uniform (PSF)	0 to 23' 8 1/2" (Front)	14' 6"	20.0	35.0	Default Load

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Page 4 / 11

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641 WARREN AVE.
SCOTSDALE, GA 30079



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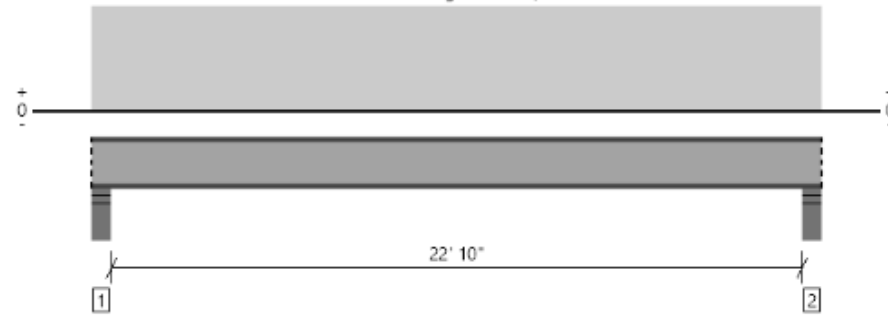
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Second Floor Framing Plan, Beam "B" (Steel Option Flush Beam)
1 piece(s) W12X40 (A992) ASTM Steel

Overall Length: 23' 8 1/2"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	11143 @ 3 3/4"	17872 (5.25")	Passed (62%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	10732 @ 5 1/4"	70210	Passed (15%)	--	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	62609 @ 11' 10 1/4"	79608	Passed (79%)	--	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.431 @ 11' 10 1/4"	0.769	Passed (L/643)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.674 @ 11' 10 1/4"	1.154	Passed (L/411)	--	1.0 D + 1.0 L (All Spans)

System : Floor
 Member Type : Flush Beam
 Building Use : Residential
 Building Code : IBC 2018
 Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Applicable calculations are based on ANSI/AISC 360-16.
- A lateral-torsional buckling factor (C_b) of 1.0 has been assumed.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Total	
1 - Stud wall - SPF	5.25"	5.25"	5.25"	4030	7113	11143	Blocking
2 - Stud wall - SPF	5.25"	5.25"	5.25"	4030	7113	11143	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	End Bearing Points	
Bottom Edge (Lu)	End Bearing Points	

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Comments
0 - Self Weight (PLF)	0 to 23' 8 1/2"	N/A	40.0	--	
1 - Uniform (PSF)	0 to 23' 8 1/2"	15'	20.0	40.0	Default Load

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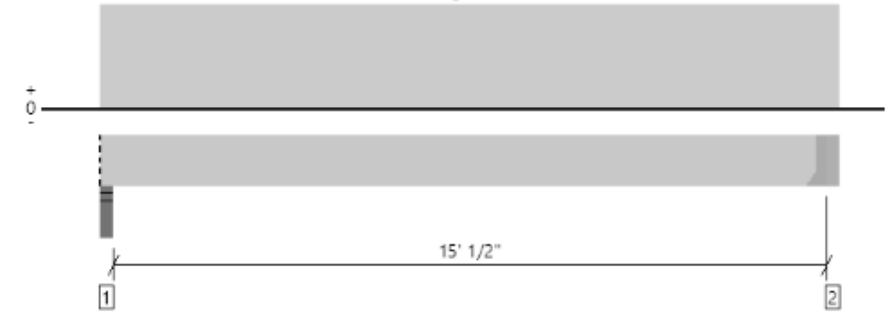
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 Page 5 / 11

Second Floor Framing Plan, Joist "B"
1 piece(s) 2 x 12 SPF No.1/No.2 @ 16" OC

Overall Length: 15' 7 1/2"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	605 @ 15' 4"	956 (1.50")	Passed (63%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	530 @ 14' 4 3/4"	1519	Passed (35%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	2288 @ 7' 9 1/4"	2653	Passed (86%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.252 @ 7' 9 1/4"	0.504	Passed (L/720)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.378 @ 7' 9 1/4"	0.756	Passed (L/480)	--	1.0 D + 1.0 L (All Spans)
TJ-Pro™ Rating	N/A	N/A	N/A	--	N/A

System : Floor
 Member Type : Joist
 Building Use : Residential
 Building Code : IBC 2018
 Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- A 15% increase in the moment capacity has been added to account for repetitive member usage.
- Applicable calculations are based on NDS.
- No composite action between deck and joist was considered in analysis.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Total	
1 - Stud wall - SPF	3.50"	3.50"	1.50"	207	414	621	Blocking
2 - Hanger on 11 1/4" LVL beam	3.50"	Hanger ¹	1.50"	209	419	628	See note ¹

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

• At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger

• ¹ See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	4' 4" o/c	
Bottom Edge (Lu)	15' 4" o/c	

• Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie						
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
2 - Face Mount Hanger	LUS28	1.75"	N/A	6-10dx1.5	3-10d	

• Refer to manufacturer notes and instructions for proper installation and use of all connectors.

Vertical Load	Location (Side)	Spacing	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform (PSF)	0 to 15' 7 1/2"	16"	20.0	40.0	Default Load

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 Page 6 / 11

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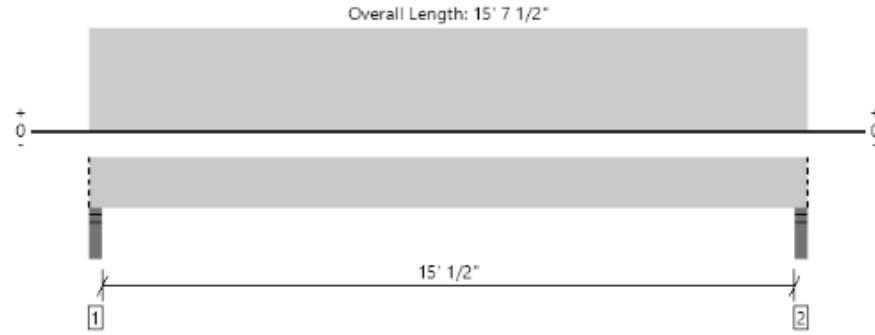
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Second Floor Ceiling Framing Plan, Joist "C"
1 piece(s) 2 x 8 SPF No.1/No.2 @ 16" OC



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDf	Load: Combination (Pattern)
Member Reaction (lbs)	281 @ 2 1/2"	2231 (3.50")	Passed (13%)	--	1.0 D + 1.0 Lr (All Spans)
Shear (lbs)	249 @ 10 3/4"	1223	Passed (20%)	1.25	1.0 D + 1.0 Lr (All Spans)
Moment (Ft-lbs)	1041 @ 7' 9 3/4"	1653	Passed (63%)	1.25	1.0 D + 1.0 Lr (All Spans)
Live Load Defl. (in)	0.361 @ 7' 9 3/4"	0.760	Passed (L/506)	--	1.0 D + 1.0 Lr (All Spans)
Total Load Defl. (in)	0.650 @ 7' 9 3/4"	1.014	Passed (L/281)	--	1.0 D + 1.0 Lr (All Spans)

- Deflection criteria: LL (L/240) and TL (L/180).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- A 15% increase in the moment capacity has been added to account for repetitive member usage.
- Applicable calculations are based on NDS.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Roof Live	Total	
1 - Stud wall - SPF	3.50"	3.50"	1.50"	125	156	281	Blocking
2 - Stud wall - SPF	3.50"	3.50"	1.50"	125	156	281	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	8' 4" o/c	
Bottom Edge (Lu)	15' 8" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Load	Location (Side)	Spacing	Dead (0.90)	Roof Live (non-snow: 1.25)	Comments
1 - Uniform (PSF)	0 to 15' 7 1/2"	16"	12.0	15.0	Default Load

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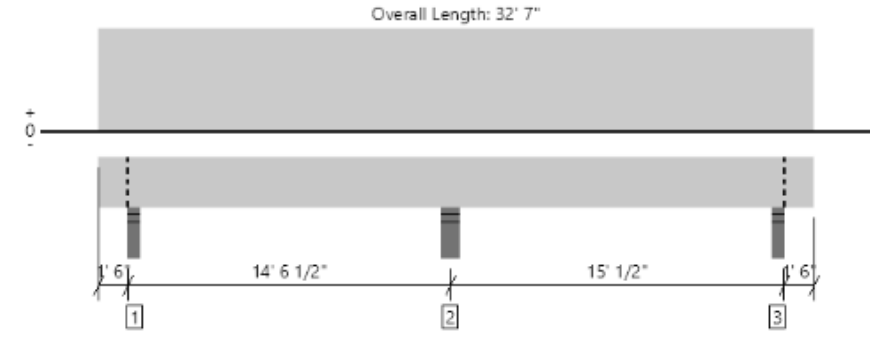
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 Page 7 / 11

Second Floor Roof Framing Plan, Ridge Beam "C"
2 piece(s) 1 3/4" x 9 1/2" 2.0E Microllam® LVL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDf	Load: Combination (Pattern)
Member Reaction (lbs)	7422 @ 16' 1/2"	7809 (5.25")	Passed (95%)	--	1.0 D + 1.0 Lr (Adj Spans)
Shear (lbs)	3336 @ 17' 5/8"	7897	Passed (42%)	1.25	1.0 D + 1.0 Lr (Adj Spans)
Moment (Ft-lbs)	-10798 @ 16' 1/2"	14719	Passed (73%)	1.25	1.0 D + 1.0 Lr (Adj Spans)
Live Load Defl. (in)	0.320 @ 24' 1 13/16"	0.745	Passed (L/558)	--	1.0 D + 1.0 Lr (Alt Spans)
Total Load Defl. (in)	0.493 @ 24' 3 7/16"	0.993	Passed (L/363)	--	1.0 D + 1.0 Lr (Alt Spans)

- Deflection criteria: LL (L/240) and TL (L/180).
- Overhang deflection criteria: LL (2L/240) and TL (2L/180).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Roof Live	Total	
1 - Stud wall - SPF	3.50"	3.50"	2.04"	1208	1827	3035	Blocking
2 - Stud wall - SPF	5.25"	5.25"	4.99"	3060	4361	7421	None
3 - Stud wall - SPF	3.50"	3.50"	2.11"	1261	1885	3146	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	17' o/c	
Bottom Edge (Lu)	9' 11" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Comments
0 - Self Weight (PLF)	0 to 32' 7"	N/A	9.7	--	
1 - Uniform (PSF)	0 to 32' 7" (Front)	16'	10.0	15.0	Default Load

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 Page 8 / 11

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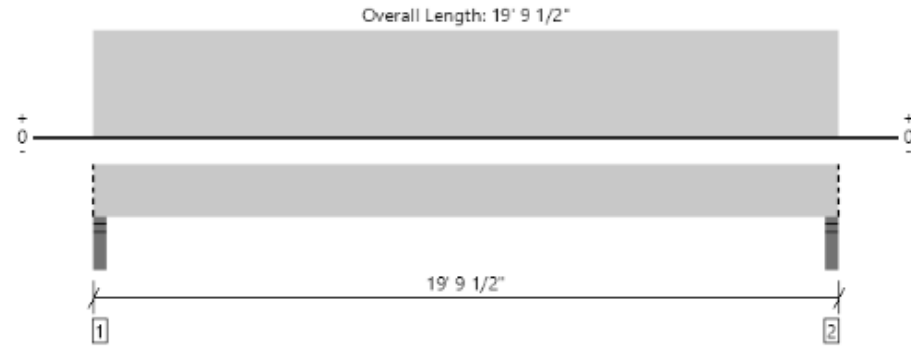
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Second Floor Roof Framing Plan, Ridge Beam "D"
1 piece(s) 1 3/4" x 11 7/8" 2.0E Microllam® LVL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1792 @ 2"	2603 (3.50")	Passed (69%)	--	1.0 D + 1.0 Lr (All Spans)
Shear (lbs)	1560 @ 1' 3 3/8"	4936	Passed (32%)	1.25	1.0 D + 1.0 Lr (All Spans)
Moment (Ft-lbs)	8569 @ 9' 10 3/4"	11155	Passed (77%)	1.25	1.0 D + 1.0 Lr (All Spans)
Live Load Defl. (in)	0.721 @ 9' 10 3/4"	0.973	Passed (L/324)	--	1.0 D + 1.0 Lr (All Spans)
Total Load Defl. (in)	1.243 @ 9' 10 3/4"	1.297	Passed (L/188)	--	1.0 D + 1.0 Lr (All Spans)

- Deflection criteria: LL (L/240) and TL (L/180).
- Allowed moment does not reflect the adjustment for the beam stability factor.

System : Roof
 Member Type : Flush Beam
 Building Use : Residential
 Building Code : IBC 2018
 Design Methodology : ASD
 Member Pitch : 0/12

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Roof Live	Total	
1 - Stud wall - SPF	3.50"	3.50"	2.41"	753	1039	1792	Blocking
2 - Stud wall - SPF	3.50"	3.50"	2.41"	753	1039	1792	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	4' 4" o/c	
Bottom Edge (Lu)	19' 10" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Comments
0 - Self Weight (PLF)	0 to 19' 9 1/2"	N/A	6.1	--	
1 - Uniform (PSF)	0 to 19' 9 1/2" (Front)	7'	10.0	15.0	Default Load

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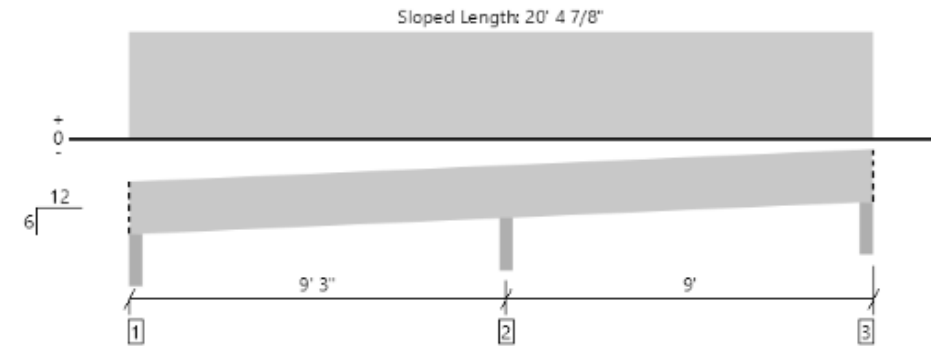
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 Page 9 / 11

Second Floor Roof Framing Plan, Rafter "A"
1 piece(s) 2 x 6 SPF No.1/No.2 @ 16" OC



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Member Length : 20' 7 5/8"

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	422 @ 9' 3"	2495 (3.50")	Passed (17%)	--	1.0 D + 1.0 Lr (All Spans)
Shear (lbs)	192 @ 8' 8 5/16"	928	Passed (21%)	1.25	1.0 D + 1.0 Lr (All Spans)
Moment (Ft-lbs)	-377 @ 9' 3"	1030	Passed (37%)	1.25	1.0 D + 1.0 Lr (All Spans)
Live Load Defl. (in)	0.073 @ 4' 3 5/8"	0.505	Passed (L/999+)	--	1.0 D + 1.0 Lr (All Spans)
Total Load Defl. (in)	0.122 @ 4' 2 7/16"	0.674	Passed (L/992)	--	1.0 D + 1.0 Lr (All Spans)

- Deflection criteria: LL (L/240) and TL (L/180).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- A 15% increase in the moment capacity has been added to account for repetitive member usage.
- Applicable calculations are based on NDS.

System : Roof
 Member Type : Joist
 Building Use : Residential
 Building Code : IBC 2018
 Design Methodology : ASD
 Member Pitch : 6/12

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Roof Live	Total	
1 - Beveled Plate - SPF	3.50"	3.50"	1.50"	65	78	143	Blocking
2 - Beveled Plate - SPF	3.50"	3.50"	1.50"	199	223	422	None
3 - Beveled Plate - SPF	3.50"	3.50"	1.50"	62	75	137	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	20' 5" o/c	
Bottom Edge (Lu)	18' 5" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Load	Location (Side)	Spacing	Dead (0.90)	Roof Live (non-snow: 1.25)	Comments
1 - Uniform (PSF)	0 to 18' 3"	16"	12.0	15.0	Default Load

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The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

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 Page 10 / 11

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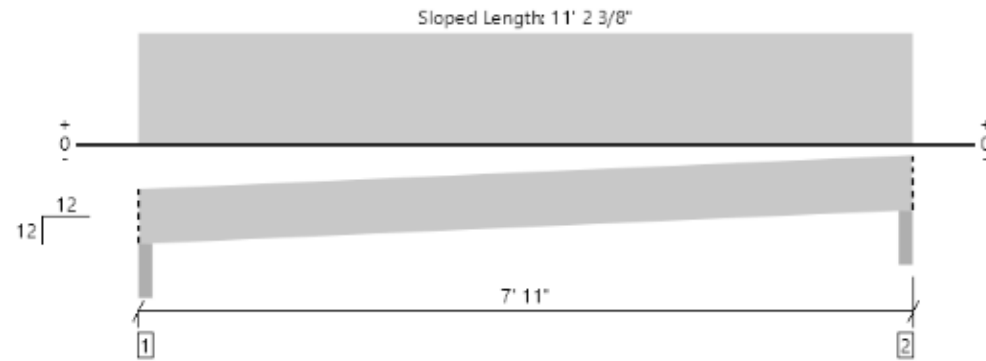
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Second Floor Roof Framing Plan, Rafter "B"
1 piece(s) 2 x 6 SPF No.1/No.2 @ 16" OC



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Member Length : 11' 7 7/8"

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	169 @ 2 1/2"	2231 (3.50")	Passed (8%)	--	1.0 D + 1.0 Lr (All Spans)
Shear (lbs)	142 @ 7 3/8"	928	Passed (15%)	1.25	1.0 D + 1.0 Lr (All Spans)
Moment (Ft-lbs)	300 @ 3' 11 1/2"	1030	Passed (29%)	1.25	1.0 D + 1.0 Lr (All Spans)
Live Load Defl. (in)	0.098 @ 3' 11 1/2"	0.530	Passed (L/999+)	--	1.0 D + 1.0 Lr (All Spans)
Total Load Defl. (in)	0.208 @ 3' 11 1/2"	0.707	Passed (L/611)	--	1.0 D + 1.0 Lr (All Spans)

System : Roof
 Member Type : Joist
 Building Use : Residential
 Building Code : IBC 2018
 Design Methodology : ASD
 Member Pitch : 12/12

- Deflection criteria: LL (L/240) and TL (L/180).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- A 15% increase in the moment capacity has been added to account for repetitive member usage.
- Applicable calculations are based on NDS.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Roof Live	Total	
1 - Beveled Plate - SPF	3.50"	3.50"	1.50"	90	79	169	Blocking
2 - Beveled Plate - SPF	3.50"	3.50"	1.50"	90	79	169	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	11' 2" o/c	
Bottom Edge (Lu)	11' 2" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Load	Location (Side)	Spacing	Dead (0.90)	Roof Live (non-snow: 1.25)	Comments
1 - Uniform (PSF)	0 to 7' 11"	16"	12.0	15.0	Default Load

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 The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

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Table 5. Fastener Spacing for a Southern Pine, Douglas Fir-Larch, or Hem-Fir Deck Ledger or Band or Rim Joist and a 2-inch Nominal Solid-Sawn Spruce-Pine-Fir Band Joist or LVL Rim Joist.^{3,4,5,6,8}
(Deck Live Load = 40 psf, Deck Dead Load = 10 psf)

Connection Details	Rim Joist or Band Joist	Joist Span						
		6'-0" and less	6'-1" to 8'-0"	8'-1" to 10'-0"	10'-1" to 12'-0"	12'-1" to 14'-0"	14'-1" to 16'-0"	16'-1" to 18'-0"
On-Center Spacing of Fasteners								
$\frac{1}{2}$ " diameter lag screw ¹ with $\frac{15}{32}$ " maximum sheathing	1" LVL	24"	18"	14"	12"	10"	9"	8"
	1- $\frac{1}{8}$ " LVL	28"	21"	16"	14"	12"	10"	9"
	1- $\frac{1}{2}$ " Lumber	30"	23"	18"	15"	13"	11"	10"
$\frac{1}{2}$ " diameter bolt with $\frac{15}{32}$ " maximum sheathing	1" LVL	24"	18"	14"	12"	10"	9"	8"
	1- $\frac{1}{8}$ " LVL	28"	21"	16"	14"	12"	10"	9"
	1- $\frac{1}{2}$ " Lumber	36"	36"	34"	29"	24"	21"	19"
$\frac{1}{2}$ " diameter bolt with $\frac{15}{32}$ " maximum sheathing and $\frac{1}{2}$ " stacked washers ^{2,7}	1- $\frac{1}{2}$ " Lumber	36"	36"	29"	24"	21"	18"	16"

- The tip of the lag screw shall fully extend beyond the inside face of the band or rim joist.
- The maximum gap between the face of the ledger board and face of the wall sheathing shall be $\frac{1}{2}$ ".
- Ledgers shall be flashed or caulked to prevent water from contacting the house band joist (see Figures 14 and 15).
- Lag screws and bolts shall be staggered per Figure 19.
- Deck ledgers shall be minimum 2x8 pressure-preservative-treated No.2 grade lumber, or other approved materials as established by standard engineering practice.
- When solid-sawn pressure-preservative-treated deck ledgers are attached to engineered wood products (minimum 1" thick wood structural panel band joist or structural composite lumber including laminated veneer lumber), the ledger attachment shall be designed in accordance with accepted engineering practice. Tabulated values based on 300 lbs and 350 lbs for 1" and 1- $\frac{1}{8}$ " LVL rim joist, respectively.
- Wood structural panel sheathing, gypsum board sheathing, or foam sheathing shall be permitted between the band or rim joist and ledger. Stacked washers are permitted in combination with wood structural panel sheathing, but are not permitted in combination with gypsum board or foam sheathing. The maximum distance between the face of the ledger board and the face of the band joist shall be 1".
- Fastener spacing also applies to Southern Pine, Douglas Fir-Larch, and Hem-Fir band or rim joists.

Figure 19. Ledger Board Fastener Spacing and Clearances.

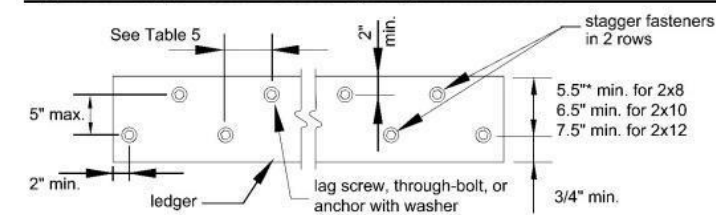


Figure 14. General Attachment of Ledger Board to Band Joist or Rim Joist.

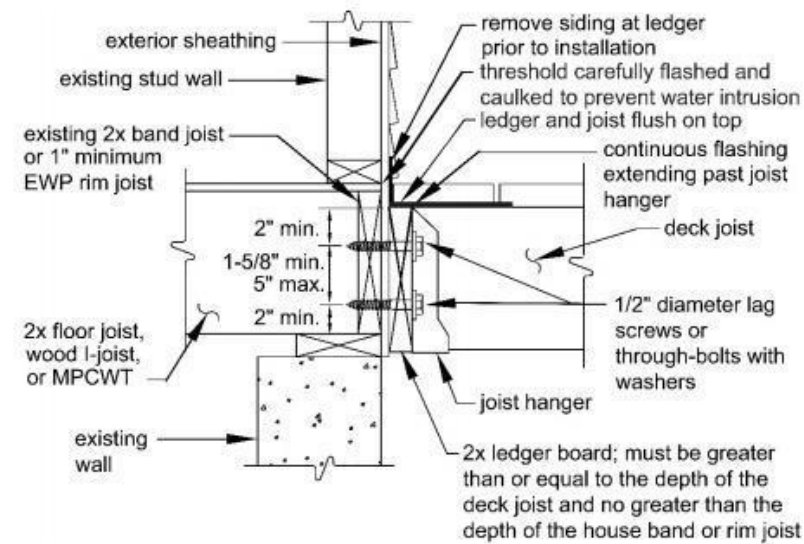


Figure 15. Attachment of Ledger Board to Foundation Wall (Concrete or Solid Masonry).

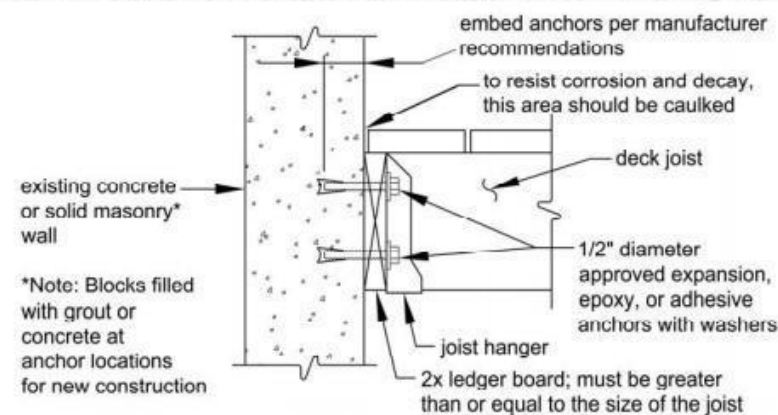
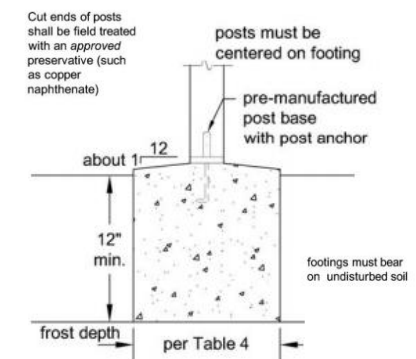


Table 4. Post Height for 6x6⁵ and Footing Sizes for all Posts.

Beam Span, L _b	Joist Span L _j	Post Heights ¹					Footing Sizes ²		
		Southern Pine	Douglas Fir-Larch ³	Hem-Fir, Western Cedars	Redwood	Ponderosa Pine, Red Pine, SPP ³	Round Footing Diameter	Square Footing	Footing Thickness ⁴
6'	≤10'	14'	14'	14'	14'	14'	18"	16"x16"	7"
	14'	14'	14'	14'	14'	14'	21"	18"x18"	8"
	18'	14'	14'	12'	14'	11'	24"	21"x21"	10"
8'	≤10'	14'	14'	14'	14'	14'	20"	18"x18"	8"
	14'	14'	14'	14'	14'	11'	24"	21"x21"	10"
	18'	14'	13'	11'	12'	8'	27"	24"x24"	11"
10'	≤10'	14'	14'	14'	14'	12'	23"	20"x20"	9"
	14'	14'	13'	11'	13'	8'	27"	24"x24"	11"
	18'	12'	11'	8'	11'	2'	31"	27"x27"	13"
12'	≤10'	14'	14'	12'	14'	10'	25"	22"x22"	10"
	14'	13'	12'	9'	11'	5'	30"	26"x26"	13"
	18'	11'	9'	6'	9'	2'	34"	30"x30"	15"
14'	≤10'	14'	13'	11'	13'	8'	27"	24"x24"	11"
	14'	11'	10'	7'	10'	2'	32"	29"x29"	14"
	18'	9'	8'	2'	8'	NP	37"	33"x33"	16"
16'	≤10'	13'	12'	10'	12'	6'	29"	26"x26"	12"
	14'	10'	9'	5'	9'	2'	35"	31"x31"	15"
	18'	7'	5'	2'	7'	NP	40"	35"x35"	18"
18'	≤10'	12'	11'	8'	11'	2'	31"	27"x27"	13"
	14'	9'	8'	2'	8'	NP	37"	33"x33"	16"
	18'	5'	2'	2'	6'	NP	42"	37"x37"	19"

- Assumes 40 psf live load, 10 psf dead load, L_b/4 and L_j/4 overhangs, No 2. grade and wet service conditions.
- Assumes 1,500 psf soil bearing capacity and 150 pcf concrete. Value may be multiplied by 0.9 for corner posts.
- Incising assumed for Douglas Fir-Larch, Hem-Fir, and Spruce-Pine-Fir.
- Assumes 2,500 psi compressive strength of concrete. Coordinate footing thickness with post base and anchor requirements.
- 8x8 nominal posts may be substituted anywhere in Table 4 to a maximum height of 14'.

Figure 12. Typical Footing



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Table 3A. Dimension Lumber Deck Beam Spans (L_B)¹ Supporting a Single Span of Joists with or without Overhangs.

Species	Size ⁴	Joist Spans (L) Less Than or Equal to:						
		6'	8'	10'	12'	14'	16'	18'
Southern Pine	2-2x6	6'-8"	5'-8"	5'-1"	4'-7"	4'-3"	4'-0"	3'-9"
	2-2x8	8'-6"	7'-4"	6'-6"	5'-11"	5'-6"	5'-1"	4'-9"
	2-2x10	10'-1"	8'-9"	7'-9"	7'-1"	6'-6"	6'-1"	5'-9"
Southern Pine	2-2x12	11'-11"	10'-4"	9'-2"	8'-4"	7'-9"	7'-3"	6'-9"
	3-2x6	7'-11"	7'-2"	6'-5"	5'-10"	5'-5"	5'-0"	4'-9"
	3-2x8	10'-7"	9'-3"	8'-3"	7'-6"	6'-11"	6'-5"	6'-1"
	3-2x10	12'-9"	11'-0"	9'-9"	8'-9"	8'-3"	7'-8"	7'-3"
	3-2x12	15'-0"	13'-0"	11'-7"	10'-6"	9'-9"	9'-1"	8'-7"

Table 2. Maximum Joist Spans and Overhangs.¹

Species	Size	Joist Spacing (o.c.)					
		12"	16"	24"	12"	16"	24"
		Allowable Span ² (L _J)			Allowable Overhang ³ (L _O)		
Southern Pine	2x6 ⁵	9'-11"	9'-0"	7'-7"	1'-0"	1'-1"	1'-3"
	2x8	13'-1"	11'-10"	9'-8"	1'-10"	2'-0"	2'-4"
	2x10	16'-2"	14'-0"	11'-5"	3'-1"	3'-5"	2'-10"
	2x12	18'-0" ⁷	16'-6"	13'-6"	4'-6"	4'-2"	3'-4"

Figure 1A. Joist Span – Joists Attached at House and Bearing Over Beam.

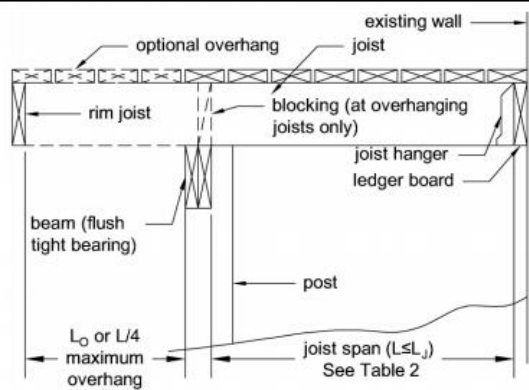


Figure 8A. Post-to-Beam Attachment Requirements.

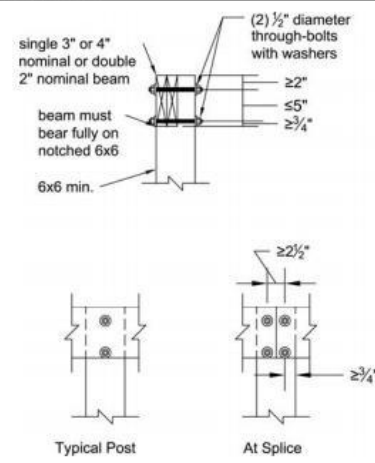


Figure 10. Diagonal Bracing.

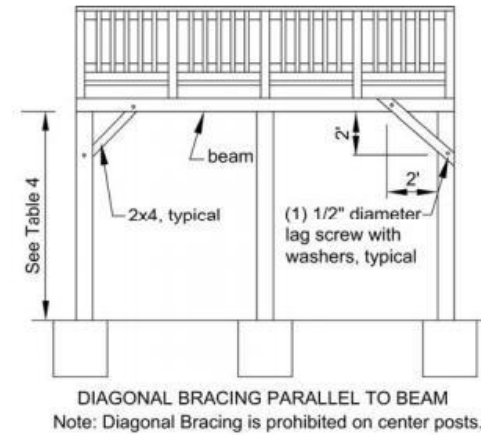


Figure 24. Example Guard Detail.

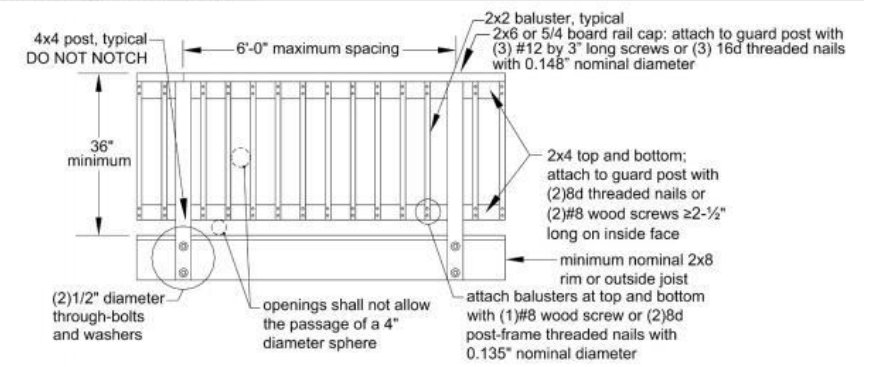


Figure 26. Guard Post to Rim Joist Example.

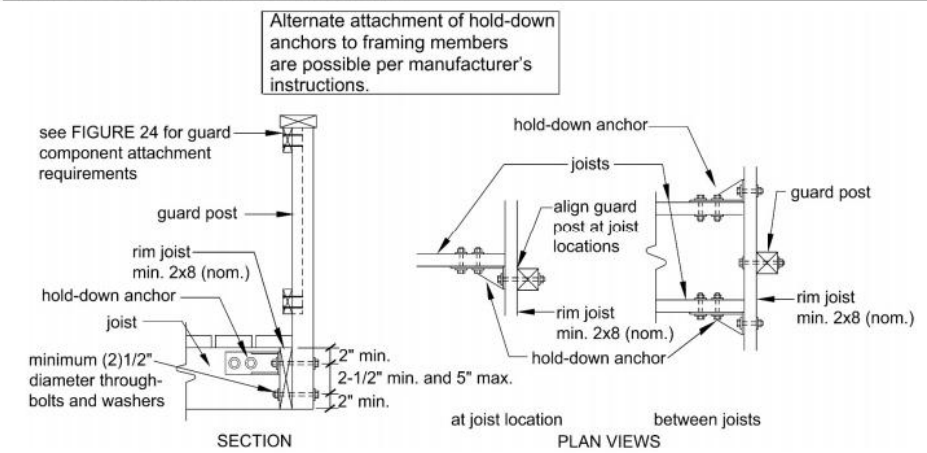


Figure 11. Rim Joist Connection Details.

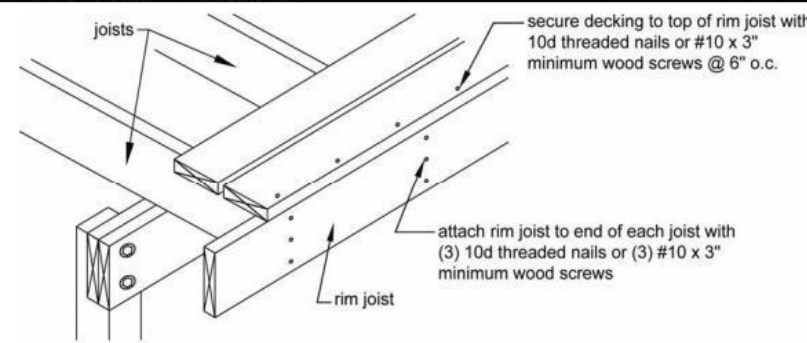


Figure 6. Joist-to-Beam Detail.

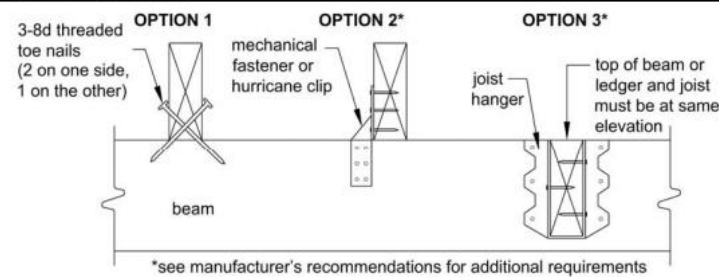


Table 3A. Joist Hanger Vertical Capacity.

Joist Size	Minimum Capacity, lbs
2x6	400
2x8	500
2x10	600
2x12	700

Figure 34. Stair Footing Detail.

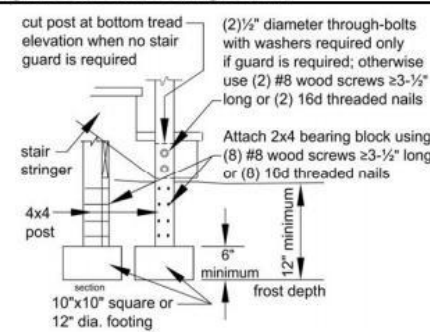


Figure 27. Tread and Riser Detail.

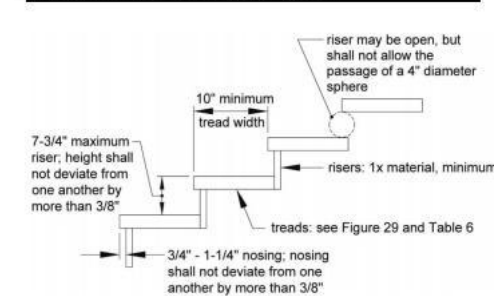
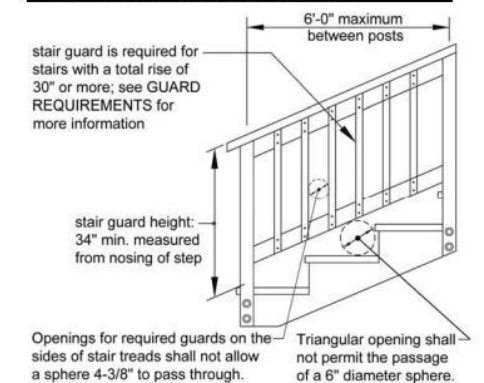


Figure 30. Stair Guard Requirements.



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