

		2869 4th St
		2392
CC	06-12-23 Direct Construction	CRAWLSPACE
1017	ENGINEERED INSPECTIONS	\$500.00
1018	INITIAL SURVEY	\$1,200.00
1020	BUILDING PERMIT	
1021	WATER METER	
1023	SEWER SPETIC PERMIT	
1040	PERMIT BOX	\$50.00
2010	CLEAR/GRADE	\$7,500.00
2011	BACKFILL FOUNDATION	\$1,000.00
2015	DRIVEWAY PAD & GRAVEL	\$650.00
2520	FOOTING LABOR	\$2,050.50
2528	TERMITE BOND	\$250.00
2550	MONOSLAB LABOR	\$0.00
2556	POURED WALL LABOR	\$19,890.00
2559	FOUNDATION WATERPROOFING	\$5,700.00
2560	SLAB PUMP, K-VEYOR	\$3,000.00
2565	FOUNDATION BLOCK LABOR	\$0.00
2569	FLATWORK LABOR	\$5,500.00
3010	FRAME MATERIAL 1ST FLOOR	\$7,550.00
3011	FRAME MAT 2ND FLOOR & ROOF	\$5,050.00
3012	ROOF / FRAME FILL-IN MATERIAL	\$3,150.00
3013	FRAMING LABOR 1ST DRAW	\$11,350.00
3014	FRAMING LABOR 2ND DRAW	\$1,300.00
3016	SUB FLOOR SCREW DOWN	\$300.00
3018	PREBUILT STAIRS	\$650.00
3020	ROOF TRUSSES	\$6,850.00
3021	1ST FLOOR TRUSSES	\$6,000.00
3022	2ND FLOOR TRUSSES	\$5,600.00
3029	CRANE RENTAL	\$600.00
3032	DRIP EDGE	\$500.00
3040	ROOFING LABOR OR TURNKEY	\$5,400.00
3046	HOUSEWRAP TURNKEY	\$1,025.00
3051	SIDING/CORNICE MATERIAL	\$10,500.00
3060	SIDING LABOR OR TURNKEY	\$5,800.00
3070	WINDOWS	\$4,600.00
3073	EXTERIOR DOORS	\$1,900.00
3075	INSTALL DOORS & WINDOW	\$1,125.00
3090	GARAGE DOOR	\$0.00
3510	PLUMBING-SLAB-DRAW 1	\$4,500.00
3511	PLUMBING-ROUGH-DRAW 2	\$4,500.00
3512	PLUMBING-FINAL-DRAW 3	\$5,950.00
3513	WATER/SEWER LINES	\$1,000.00
3515	HVAC - ROUGH	\$7,400.00
3516	HVAC - FINAL	\$4,950.00
3520	ELECTRICAL - ROUGH	\$6,100.00
3521	ELECTRICAL - FINAL	\$4,050.00
3599	DUCT BLAST/BLOWER DOOR	\$350.00
4010	FIREPLACE	\$1,395.00
4020	INSULATION - BATTS	\$3,350.00
4021	INSULATION - BLOWN	\$2,750.00
4025	INSULATION - PREHANG	\$274.50
4030	DRYWALL	\$11,100.00
4040	PAINT - EXTERIOR	\$4,900.00
4041	PAINT - INTERIOR	\$6,250.00
4042	PAINT - FINAL TOUCH UP	\$0.00
4060	TILE LABOR OR TURNKEY	\$1,500.00
6001	INTERIOR TRIM MATERIAL	\$4,850.00
6003	DOOR HARDWARE	\$725.00
6004	SHOE MOULD	\$175.00
6005	VENTILATED SHELVING	\$575.00
6006	MANTEL & HEARTH	\$600.00
6010	INTERIOR TRIM LABOR	\$3,100.00
6011	LOCKOUT LABOR	\$300.00
6012	INTERIOR DOORS	\$0.00
6021	MIRRORS	\$900.00
6023	BATH ACCESSORIES	\$135.00
6024	SHOWER DOOR	\$600.00
6049	GRANITE TOPS	\$3,900.00
6050	CABINETS	\$7,500.00
6051	VANITY TOPS	\$1,002.46
6054	CERAMIC TILE BACKSPLASH	\$550.00
6070	BUILT INS	\$0.00
6080	LIGHT FIXTURES	\$1,800.00
6090	APPLIANCES	\$2,750.00
7030	PORCH COLUMNS & RAILING	\$0.00
7031	FRONT PORCH LABOR OR TURNKEY	\$300.00
7047	FLOORING - VINYL	\$10,800.00
7049		\$0.00
7050	FLOORING - CARPET	\$1,500.00
7060	HOUSE CLEAN 1 - ROUGH	\$250.00
7061	HOUSE CLEAN 2 - INTERIOR	\$0.00

		2869 4th St
		2392
CC	Direct Construction	CRAWLSPACE
7062	HOUSE CLEAN 3 - FINAL	\$500.00
7063	HOUSE CLEAN 4 - TOUCH UP	\$400.00
7065	GUTTERS & SPLASH BLOCK	\$1,550.00
7066	METAL ROOF AND BAY TOPS	\$2,500.00
7069	FINAL GRADE	\$600.00
7070	LANDSCAPING	\$4,600.00
7072	REATAINING WALLS	\$0.00
7075	ROLL OFF CONTAINER	\$1,750.00
7077	TRASH 1	\$500.00
7084	PUNCHOUT LABOR	\$1,500.00
7086	PUNCH OUT MATERIALS	\$0.00
7088	MAIL BOX / ADDRESS PLAQUE	\$250.00
7089	SILT FENCE-LAB/MATERIAL	\$2,000.00
7090	UTILITIES	\$600.00
7092	WHEAT STRAW	\$0.00
7095	PRESSURE WASH	\$300.00
7553	BLINDS	\$1,320.00
7599	FENCE	\$0.00
9001	BUILDER FEE	\$0.00
9020	DECORATOR FEE	\$450.00
9050	PAYMENT EXTRAS RECEIVED	\$0.00
9055	BUILDER FEE	\$25,000.00
9065	ARCHITECTURE/PLANS	
9075	BUILDERS RISK	\$350.00
9076	GENERAL LIAB INSURANCE	\$350.00
9077	WORKERS COMP INSURANCE	\$0.00
9080	REAL ESTATE TAXES	
9092	RESTROOM RENTALS	\$690.00
9206	COMMISSION	\$0.00
9230	CLOSING COSTS	\$0.00
9280	ACCOUNTING FEES INITIAL	\$1,250.00
9281	ACCOUNTING FEES FINAL	\$1,250.00
9285	MANAGEMENT FEE	\$0.00
9290	INTERIM FINANCING COSTS	\$0.00
9294	LOT INTEREST	\$0.00
9415	CONST LOAN INTEREST	\$0.00
9416	CONST LOAN INSPECTIONS	\$0.00
9420	LOT CLOSING COSTS	\$0.00
9801	THEFT REPLACEMENT	\$0.00
9804	MISCELLANOUS REPAIRS	\$0.00
9806	CARPET CLEANING	\$0.00
9810	SPEC HOME MAINTENANCE	\$0.00
9988		\$0.00
--- Grand Totals --->		\$286,882.46
Lot Cost		\$0.00
Direct Construction Costs		\$257,542.46
Indirect Costs		\$29,340.00
Financing Costs		\$0.00
Base Sales Price		\$0.00
Gross Margin (before overhead)		-\$286,882.46
Gross Margin %		
Direct Const Cost/ Sq. Ft.		\$107.12
Total Cost/ Sq. Ft.		\$119.93
Sales Price/ Sq. Ft.		\$0.00

NEW CONSTRUCTION

SCOPE OF WORK:

New construction of 4/3 SFR on crawlspace, with front porch and rear deck.

2869 4th Street
Atlanta, GA 30315

SQUARE FOOTAGE:

Proposed First Floor: 1124 sf
Proposed Second Floor: 997 sf
Proposed Porch: 100 sf
Proposed Deck: 171 sf

PROPOSED TOTAL: 2420 sf

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Label	Title
A0	Cover Sheet
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A1.2	Proposed Elevations
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S1.1	Foundation Plan
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S1.13	Load Calculations

29'-1 1/2"



Current Mandatory Codes as Adopted by DCA:

- International Building Code, 2018 Edition, with Georgia Amendments (2020), (2022)
- International Residential Code, 2018 Edition, with Georgia Amendments (2020)
- International Fire Code, 2018 Edition (Contact State Fire Marshal Below)
- International Plumbing Code, 2018 Edition, with Georgia Amendments (2020), (2022), (2023)
- International Mechanical Code, 2018 Edition, with Georgia Amendments (2020)
- International Fuel Gas Code, 2018 Edition, with Georgia Amendments (2020), (2022)
- National Electrical Code, 2020 Edition, with Georgia Amendments (2021)
- International Energy Conservation Code, 2015 Edition, with Georgia Supplements and Amendments (2020), (2022), (2023)
- International Swimming Pool and Spa Code, 2018 Edition, with Georgia Amendments (2020)
- For information and questions regarding the Life Safety Code (NFPA 101), IFC Georgia Amendments or the Georgia Accessibility Code please contact the State Fire Marshal's Office.

NUMBER	DATE	REVISION	DESCRIPTION

RELEASED FOR CONSTRUCTION

NEW CONSTRUCTION
2869 4th Street
Atlanta, GA 30315



DATE:
3/16/2023

SCALE:

SHEET:
A0



NEW CONSTRUCTION

2869 4th Street
Atlanta, GA 30315

CONSTRUCTION AND FRAMING NOTES

1. DESIGN LOADS ARE AS FOLLOWS PER SQ. FT.

LOCATION	LIVE	DEAD	DEFLECT LIMIT
1 ST FLOOR	40 LB	10 LB	L/360
2 ND FLOOR (SLEEPING AREA)	30 LB	10 LB	L/360
ATTIC (NON STORAGE)	10 LB	5LB	L/240
ATTIC (STORAGE)	20 LB	10 LB	L/240
ROOF (W/ FINISHED CEILING)	30 LB	15 LB	L/240
ROOF(NO FINISHED CEILING)	30 LB	7LB	L/180
DECKS	60 LB	10LB	L/360

SNOW LOADS HAVE BEEN ADJUSTED TO REFLECT THE SLIDE OFF FACTOR AS A FUNCTION OF ROOF PITCH. RAFTER SIZES MAY HAVE TO BE INCREASED TO ACCOMMODATE HIGHER SNOW LOADS. VERIFY WITH LOCAL CODES.

2. LUMBER SHALL BE DOUGLAS-FIR-LARCH, HEM-FIR, OR SOUTHERN YELLOW PINE WITH FB=1450 AND E=1.6 MINIMUM.

ALL PRESSURE TREATED LUMBER WILL BE A MINIMUM OF SYP#2 WITH A MOISTURE CONTENT OF 19%

3. ALL HEADERS SHALL BE FREE FROM ALL SPLITS, CHECKS, OR SHAKES.

4. UNLESS NOTED OTHERWISE, PROVIDE DOUBLE HEADER JOISTS AND TRIMMERS AT ALL FLOOR OPENINGS, DOUBLE JOISTS UNDER ALL PARALLEL PARTITIONS, DOUBLE 2X12 HEADERS WITH 1/2" PLYWOOD, GLUED BETWEEN AND NAILED, FOR ALL OPENINGS IN 2X6 WALLS. DOUBLE 2X12 HEADERS NAILED TOGETHER FOR ALL OPENINGS IN 2X4 WALLS.

5. FLOOR CONSTRUCTION: 3/4" TONGUE AND GROOVE SUBFLOOR WITH FINISH MATERIAL OVER.

6. STAIR CONSTRUCTION SHALL CONSIST OF (3) 2X2 STRINGERS, 5/4" OR 2X THICK TREADS AND 3/4" THICK RISERS OR MATERIALS FABRICATED BY A COMPONENT MANUFACTURER.

7. ALL WOOD PLATES IN CONTACT WITH CONCRETE SHALL BE PRESSURE TREATED AND SILICONE SEALED.

8. MICRO-LAM BEAMS SHALL HAVE BENDING STRESS: FB=2800 PSI. VERIFY WITH LOCAL CODES.

9. SPECIAL UPLIFT CONNECTORS AS INDICATED AT CANTILEVERED JOISTS SHALL BE SIMPSON STRONG TIE ANCHORS OR EQUAL.

10. MINIMUM HEADER SIZE SHALL BE (2) 2"x6" UNLESS NOTED OTHERWISE EXTERIOR WALLS SHALL BE (2) 2X12 WITH 1/2" PLYWOOD.

11. ALL STRUCTURAL STEEL SHALL CONFORM WITH ASTM SPECIFICATION A-36.

12. UNLESS OTHERWISE NOTED, PROVIDE A 2X PLATE BOLTED TO THE TOP FLANGE OF ALL STEEL BEAMS WITH 3/8" DIAMETER BOLTS STAGGERED AT 24" ON CENTER. RIGIDLY FASTEN ALL CONNECTING RAFTERS AND JOISTS AS APPROVED BY GOVERNING CODES, UNLESS OTHERWISE NOTED.

13. FLOOR FRAMING LAYOUT SHALL BE COORDINATED WITH THE GENERAL AND HVAC CONTRACTORS TO PROVIDE ACCESS CHASES AND UNOBSTRUCTED RUNS FOR HVAC DUCT WORK. FLOOR TRUSS LAYOUT TO BE ENGINEERED BY TRUSS MANUFACTURE.

14. PROVIDE BRIDGING OR BLOCKING AT MIDSPAN OF JOISTS/RAFTERS/TRUSSES. MAXIMUM SPACING BETWEEN BEARING WALL AND BLOCKING IS 8'0".

15. THESE FRAMING PLANS WERE DESIGNED USING STANDARD CONSTRUCTION PRACTICES. THEY CONFORM TO STANDARD BUILDING CODES. DUE TO VARIATIONS IN LOCAL CODES AND GEOLOGICAL CONDITIONS REVISIONS MAY BE REQUIRED TO THESE PLANS.

16. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE LOCAL CODES. REGULATIONS AND FHA/VA MPS. THE BUILDER SHALL VERIFY ALL CONDITIONS WITH LOCAL STRUCTURAL ENGINEERS AND CODE OFFICIALS PRIOR TO USING THE FRAMING MATERIALSPROVIDED TO INSURE COMPLIANCE WITH CODES AND STRUCTURAL INTEGRITY.

17. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI

NOTE:

1. HVAC TO BE IN ATTIC. VERIFY WITH BUILDER.

2. UPPER FLOOR CEILING HEIGHTS TO BE 8'0" UNLESS NOTED.

3. UPPER FLOOR JOISTS TO BE 16 1/2" OPEN WEB FLOOR.

TRUSSES AT 16"OC W/3/4 T&G ADVANTECH FLOOR GLUED AND SCREWED. SEE TRUSS MANUFACTURER FOR FLOOR TRUSS LAYOUTS. ALL OPEN WEB FLOOR TRUSSES TO BE DESIGNED AND ENGINEERED BY TRUSS MANUFACTURER. TRUSS MANUFACTURER WILL PROVIDE TRUSS LAYOUT BASED ON ENGINEERING.

TRUSS MANUFACTURER TO SUPPLY TRUSSES WITH CHAMFERED END ON SELECTED UNITS TO ALLOW FOR EXTERIOR WALL SUPPORT AND RATER CLEARANCE.

4. HVAC AND WATER HEATER TO BE IN ATTIC UNLESS OTHERWISE NOTED.

ELEVATION NOTES:

1. GUTTERS AND DOWNSPOUTS ARE NOT SHOWN FOR CLARITY. DOWNSPOUTS SHALL BE LOCATED TOWARDS THE FRONT AND REAR OF THE HOUSE. LOCATE DOWNSPOUTS IN NON-VISUALLY OFFENSIVE LOCATIONS. FOR EXAMPLE, FRONT WALL OF HOUSE BESIDE PORCH COLUMNS, ETC. GENERAL CONTRACTOR SHALL VERIFY EXISTING GRADES AND COORDINATE ANY NECESSARY ADJUSTMENTS TO HOUSE WITH OWNER.

2. PLUMBING AND HVAC VENTS SHALL BE GROPED IN ATTIC TO LIMIT ROOF PENETRATIONS AND TO BE LOCATED AWAY FROM PUBLIC VIEW. I.E. AT THE REAR OF THE HOUSE AND SHALL BE PRIMED AND PAINTED TO MATCH ROOF COLOR.

3. PROVIDE ATTIC VENTILATION PER LOCAL CODE REQUIREMENTS.

4. EXTERIOR FLASHING SHALL BE CORRECTLY INSTALLED AT ALL CONNECTIONS BETWEEN ROOFS, WALLS, CHIMNEYS, PROJECTIONS, AND PENETRATIONS AS REQUIRED BY APPROVED CONSTRUCTION PRACTICES.

5. CONTRACTOR SHALL PROVIDE ADEQUATE ATTIC VENTILATIONS ROOF VENTS PER LOCAL GOVERNING CODE. INSTALL CONTINUOUS RIDGE VENTILATION AND PAINT TO MATCH ROOF. PROVIDE APPROPRIATE SOFFIT VENTILATION AT OVERHANGS.

FRAMING NOTES:

1. RAFTERS TO BE SUPPORTED BY CONTINUOUS BRACING FOR HORIZONTAL SPANS OF 15'0" OR GREATER.

2. SUPPORT ALL HIP, VALLEY, AND RIDGES @ 8'0" OC MAX.

3. ALL RAFTERS TO BEAR ON SECOND FLOOR WALLS WHERE APPLICABLE.

4. RAFTERS MAY BE SPLICED ONLY @ CONT. BRACING OR SECOND FLOOR WALLS.

5. RAFTERS TO BE PLACED IN COMPLIANCE WITH ALL LOCAL CODES. EXAMPLES:

2X6 RAFTER@16"OC MAX WITH 1/2" P W DECKING

2X6 RAFTERS @ 24"OC MAX WITH 5/8"P W DECKING

2X8 RAFTERS @ 24"OC MAX WITH 5/8"P W DECKING

2X8 RAFTERS @ 16"OC MAX WITH 1/2" P W DECKING

6. FASCIA OVERHANG TO BE 12" (TYPICAL) UNLESS NOTED ON ELEVATIONS.

7. ALL HIP/VALLEY RAFTERS TO BE 2X10 UNLESS NOTED.

NOTE:

PURLINS ARE PERMITTED TO BE INSTALLED TO REDUCE THE SPAN OF RAFTERS. PURLINS SHALL BE SUPPORTED BY 2 INCH X 4 INCH BRACES INSTALLED TO BEARING WALLS AT A SLOPE OF NOT LESS THAN 45 DEGREES. THE BRACES SHALL NOT BE SPACED MORE THAN 48" APART ON CENTER AND THE UNBRACED LENGTH OF BRACES SHALL NOT EXCEED

8 FT. PURLINS SHALL BE CONTINUOUS (REFER IRC R802.5.1)

FLOOR PLANS NOTES:

1. ALL STRUCTURAL INFORMATION SHOWN FOR REFERENCE PURPOSES ONLY. CONTRACTOR SHALL HAVE LICENSED STRUCTURAL ENGINEER REVIEW AND DESIGN ALL STRUCTURAL ELEMENTS SUCH AS ALL FRAMING WALLS, BEAMS, CONNECTIONS, HEADERS, JOISTS, AND RAFTERS.

2. ALL DIMENSIONS ARE FROM FACE OF STUD TO FACE OF STUD UNLESS NOTED OTHERWISE.

3. WINDOW SIZES INDICATED ON PLANS ARE NOTED BY APPROXIMATE ROUGH OPENING SIZE; REFER TO PLANS AND EXTERIOR ELEVATIONS FOR WINDOW TYPES.

4. COORDINATE LOCATION OF UTILITY METERS WITH SITE PLAN AND LOCATE AWAY FROM PUBLIC VIEW. VISUAL IMPACT SHALL BE MINIMIZED, I.E. MOUNT AS LOW AS POSSIBLE.

5. PREFABRICATED FIREPLACE CONSTRUCTION SHALL MEET OR EXCEED ALL APPLICABLE CODES REGARDING USE OF FIRE SEPARATIONS, CLEARANCES, ETC. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT ALL ITEMS AND CONSTRUCTION MEET OR EXCEED CODE. OVERALL FLUE HEIGHT SHALL BE COORDINATED TO MATCH HEIGHT

SHOWN ON PLANS AND SHALL NOT EXCEED THE TOP OF CHIMNEY CHASES AS CONSTRUCTED.

6. CONTRACTOR SHALL COORDINATE ALL CLOSET SHELIVING REQUIREMENTS.

7. DO NOT SCALE DRAWINGS. FOLLOW DIMENSIONS.

8. CONTRACTOR SHALL FIELD VERIFY ALL CABINET DIMENSIONS BEFORE FABRICATION.

9. BEDROOM WINDOWS SHALL HAVE A MINIMUM NET CLEAR OPENING OF 5.7 SQ FT. A MINIMUM NET CLEAR OPENABLE WIDTH OF 20", A MINIMUM NET CLEAR OPENABLE HEIGHT OF 24" AND HAVE A MAXIMUM FINISH SILL HEIGHT OF 43" FROM FINISH FLOOR.

10. ALL GLASS LOCATED WITHIN 18" OF FLOOR, 12" OF A DOOR OR LOCATED WITHIN 60" OF FLOOR AT BATHTUBS, WHIRLPOOLS, SHOWERS, SAUNAS, STEAM ROOMS, OR HOT TUBS SHALL BE TEMPERED.

11. ALL EXPOSED INSULATION SHALL HAVE A FLAME SPREAD RATING OF LESS THAN 25 AND A SMOKE DENSITY RATING OF LESS THAN 450.

12. PROVIDE COMBUSTION AIR VENTS, WITH SCREEN AND BACK DAMPER, FOR FIREPLACES, WOOD STOVES AND ANY APPLIANCE WITH AN OPEN FLAME.

13. BATHROOMS AND UTILITY ROOMS SHALL BE VENTED TO THE OUTSIDE WITH A MINIMUM OF A 90 CFM FAN. RANGE HOODS SHALL ALSO BE VENTED TO OUTSIDE.

14. ATTIC HVAC UNITS SHALL BE LOCATED WITHIN 20' OF ITS SERVICE OPENING. RETURN AIR GRILLES SHALL NOT BE LOCATED WITHIN 10 FEET OF A GAS FIRED APPLIANCE.

15. ALL WALLS AND CEILINGS IN GARAGE AND GARAGE STORAGE AREAS TO HAVE 5/8 TYPE X GYPSUM BOARD WITH 1 HOUR FIRE RATING. ALL EXTERIOR DOORS IN GARAGE TO BE METAL OR SOLID CORE DOORS, INCLUDING DOORS ENTERING HEAT/COOLED PORTION OF RESIDENCE.

16. ALL FIREPLACE CHASE WALLS SHALL BE INSULATED INSIDE AND OUTSIDE. PROVIDE HORIZONTAL "DRAFT STOPS" AT EACH FLOOR LEVEL BY PACKING 6"(R-19) INSULATION BETWEEN 2X4 JOISTS.

17. ALL INTERIOR WALLS SHALL BE COVERED WITH 1/2" GYPSUM BOARD WITH METAL CORNER REINFORCING, TAPE FLOAT, AND SAND (3 COATS). USE 5/8" GYPSUM BOARD ON CEILINGS WHEN SUPPORTING MEMBERS ARE 24"OC OR GREATER. USE 1/2" GYPSUM BOARD ON CEILINGS WHEN SUPPORTING MEMBERS LESS THAN 24"OC.

18. ALL BATH AND TOILET AREA WALLS AND CEILINGS SHALL HAVE WATER RESISTANT GYPSUM BOARD.

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RELEASED FOR
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2869 4th Street
Atlanta, GA 30315



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3/16/2023

SCALE:

SHEET:

A0.1



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Atlanta, GA 30315

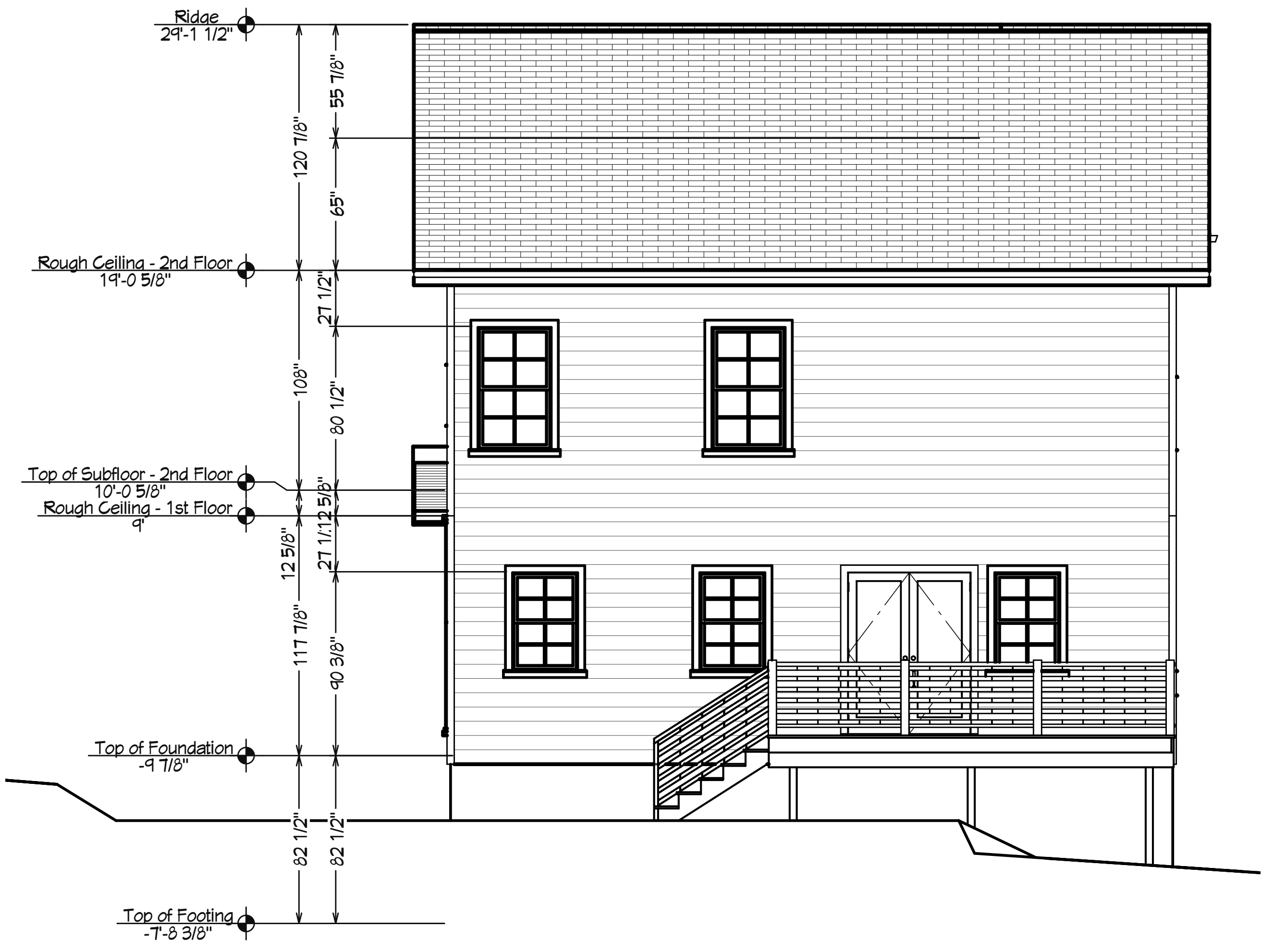


DATE:
3/16/2023

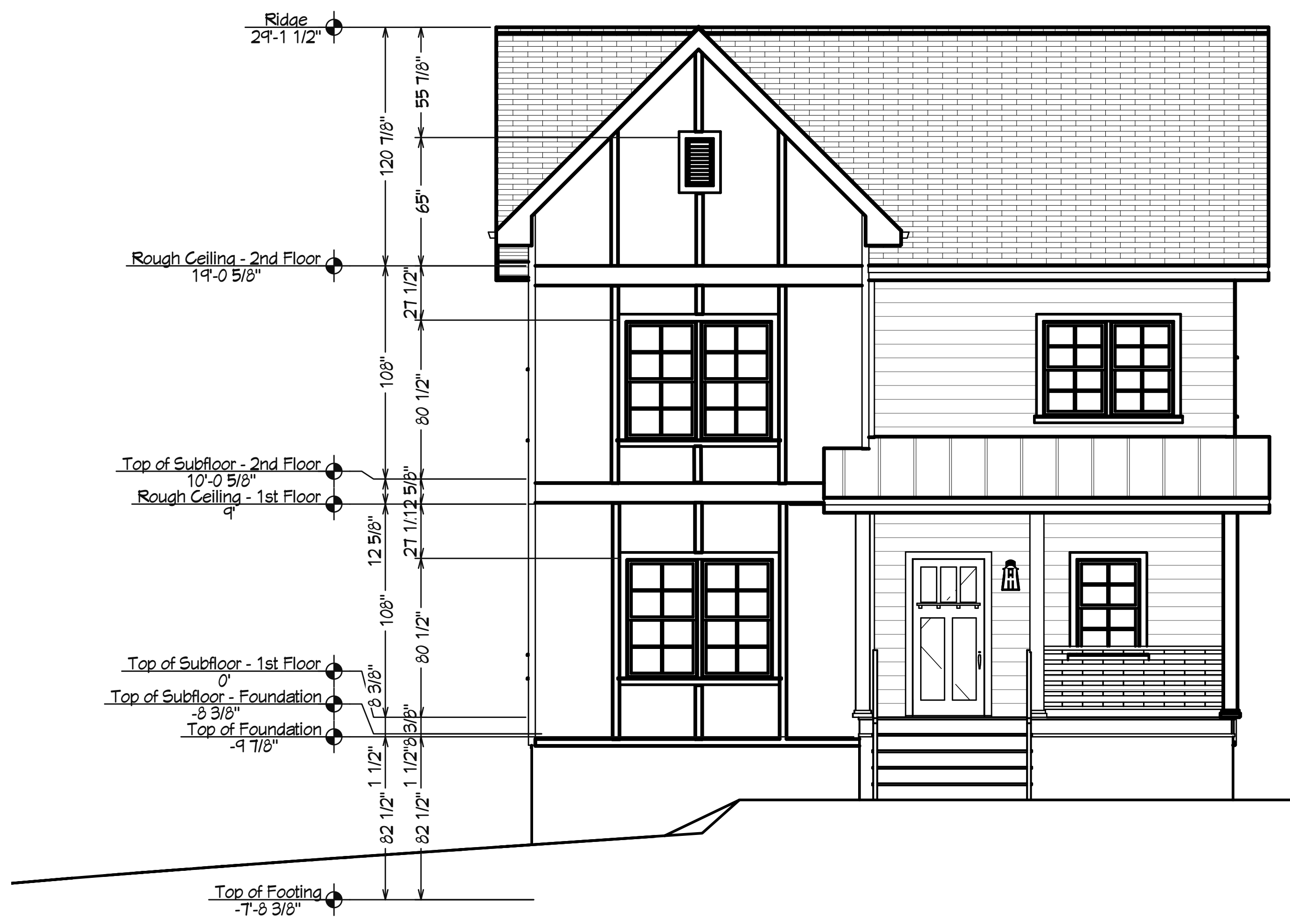
SCALE:

SHEET:

A1.1



Proposed Rear Elevation
Scale: 1/8" = 1'



Proposed Front Elevation
Scale: 1/8" = 1'

DOOR SCHEDULE											
NUMBER	LABEL	QTY	FLOOR	SIZE	WIDTH	HEIGHT	R/O	DESCRIPTION	HEADER	THICKNESS	FIRE
D01	2068	1	1	2068 L IN	24 "	80 "	26"X82 1/2"	HINGED-PANEL	2X6X29" (2)	1 3/8"	
D02	2068	3	2	2068 R IN	24 "	80 "	26"X82 1/2"	HINGED-PANEL	2X6X29" (2)	1 3/8"	
D03	2468	1	1	2468 L IN	28 "	80 "	30"X82 1/2"	HINGED-PANEL	2X6X33" (2)	1 3/8"	
D04	2468	1	2	2468 L	28 "	80 "	57 1/4"X82 1/2"	POCKET-PANEL	2X6X60 1/4" (2)	1 3/8"	
D05	2668	1	1	2668 R	30 "	80 "	61 1/4"X82 1/2"	POCKET-PANEL	2X6X64 1/4" (2)	1 3/8"	
D06	2668	2	2	2668 R	30 "	80 "	61 1/4"X82 1/2"	POCKET-PANEL	2X6X64 1/4" (2)	1 3/8"	
D07	2668	2	2	2668 R IN	30 "	80 "	32"X82 1/2"	HINGED-PANEL	2X6X35" (2)	1 3/8"	
D08	2668	1	2	2668 L	30 "	80 "	30"X80"	SHOWER-GLASS SLAB		1/2"	
D09	2668	1	2	2668 L	30 "	80 "	61 1/4"X82 1/2"	POCKET-PANEL	2X6X64 1/4" (2)	1 3/8"	
D10	2668	3	2	2668 L IN	30 "	80 "	32"X82 1/2"	HINGED-PANEL	2X6X35" (2)	1 3/8"	
D12	3068	1	1	3068 L EX	36 "	80 "	38"X83"	EXT. HINGED-DOOR E21	2X6X41" (2)	1 3/4"	
D13	4068	1	1	4068 L/R IN	48 "	80 "	50"X82 1/2"	DOUBLE HINGED-PANEL	2X8X53" (2)	1 3/8"	
D14	5068	1	1	5068 L/R EX	60 "	80 "	62"X83"	EXT. DOUBLE HINGED-DOOR F01	2X8X65" (2)	1 3/4"	
D15	5068	1	2	5068 L/R	60 "	80 "	62"X82 1/2"	4 DR. BIFOLD-PANEL	2X8X65" (2)	1 3/8"	
D16	2668	1	1	2668 L	30 "	80 "	61 1/4"X82 1/2"	POCKET-PANEL	2X6X64 1/4" (2)	1 3/8"	

WINDOW SCHEDULE											
NUMBER	LABEL	QTY	FLOOR	SIZE	WIDTH	HEIGHT	R/O	EGRESS	DESCRIPTION	HEADER	TEMPERED
W02	W1	2	1	2820FX	32 "	24 "	33"X25"		FIXED GLASS	2X6X36" (2)	
W03	W1	5	1	2840DH	32 "	48 "	33"X49"		DOUBLE HUNG	2X6X36" (2)	
W04	W1	1	1	2840DH	32 "	48 "	33"X49"	YES	DOUBLE HUNG	2X6X36" (2)	
W05	W1	3	1	3050DH	36 "	60 "	37"X61"	YES	DOUBLE HUNG	2X6X40" (2)	
W06	W1	2	2	2816FX	32 "	18 "	33"X19"		FIXED GLASS	2X6X36" (2)	
W07	W1	1	2	2820FX	32 "	24 "	33"X25"		FIXED GLASS	2X6X36" (2)	YES
W08	W1	2	2	2840DH	32 "	48 "	33"X49"		DOUBLE HUNG	2X6X36" (2)	
W09	W1	2	2	2840DH	32 "	48 "	33"X49"	YES	DOUBLE HUNG	2X6X36" (2)	
W10	W1	6	2	3050DH	36 "	60 "	37"X61"	YES	DOUBLE HUNG	2X6X40" (2)	
W11	1220	3	3	1220	14 "	24 "	15"X25"		LOUVERED	2X6X18" (2)	

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Atlanta, GA 30315



DATE:

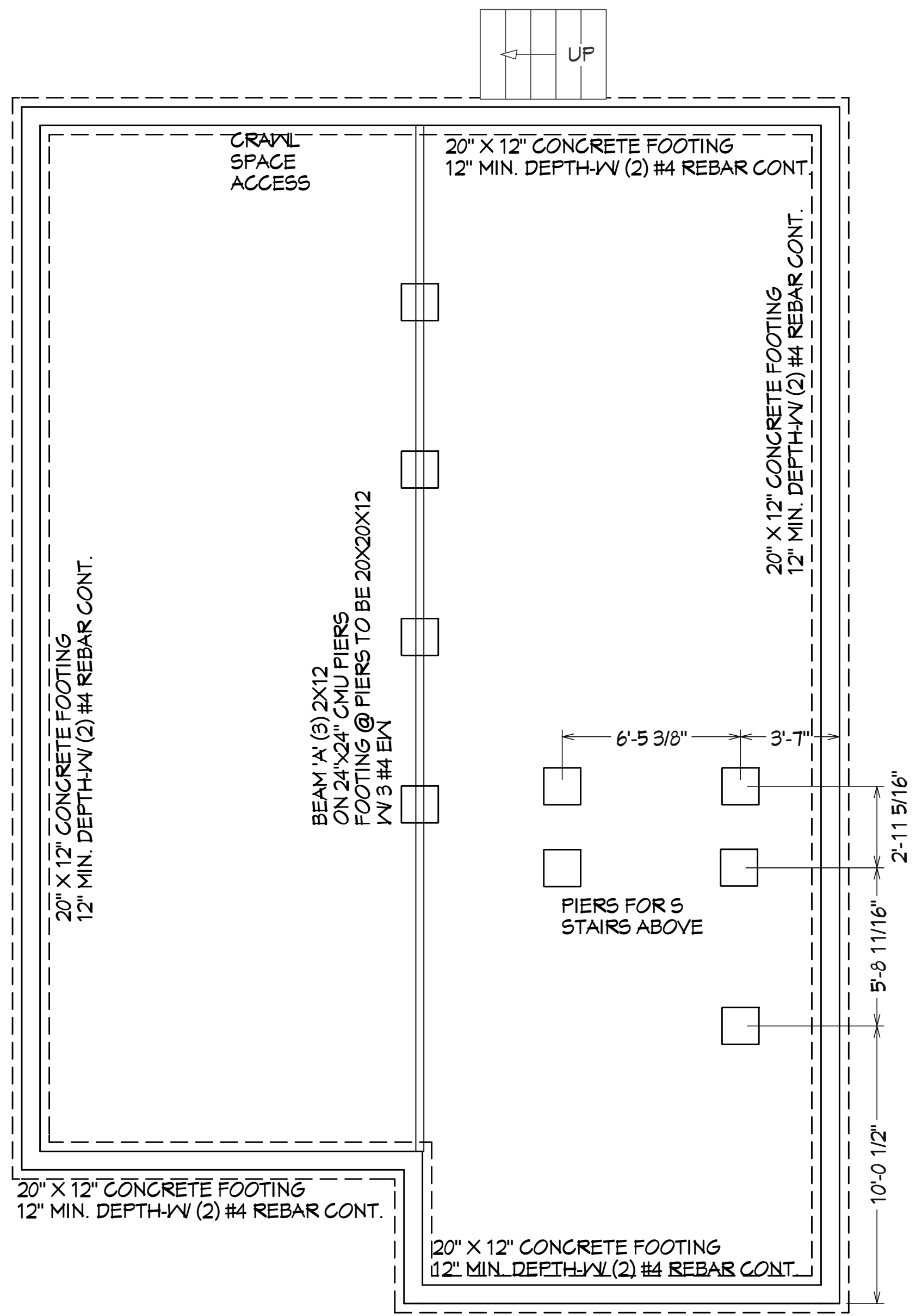
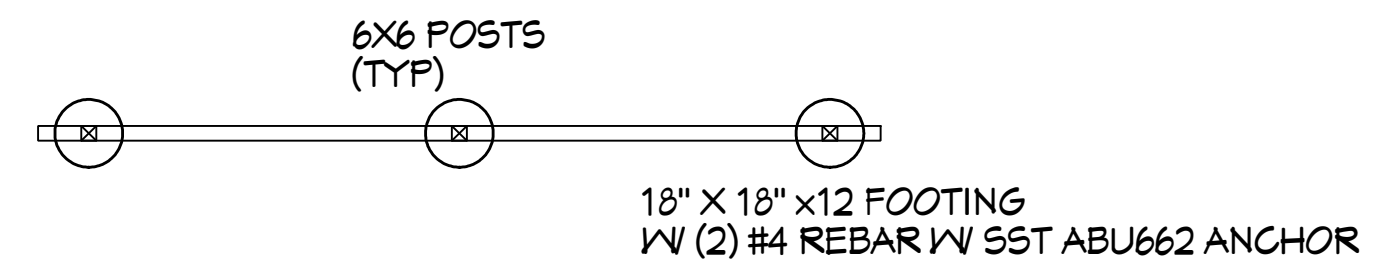
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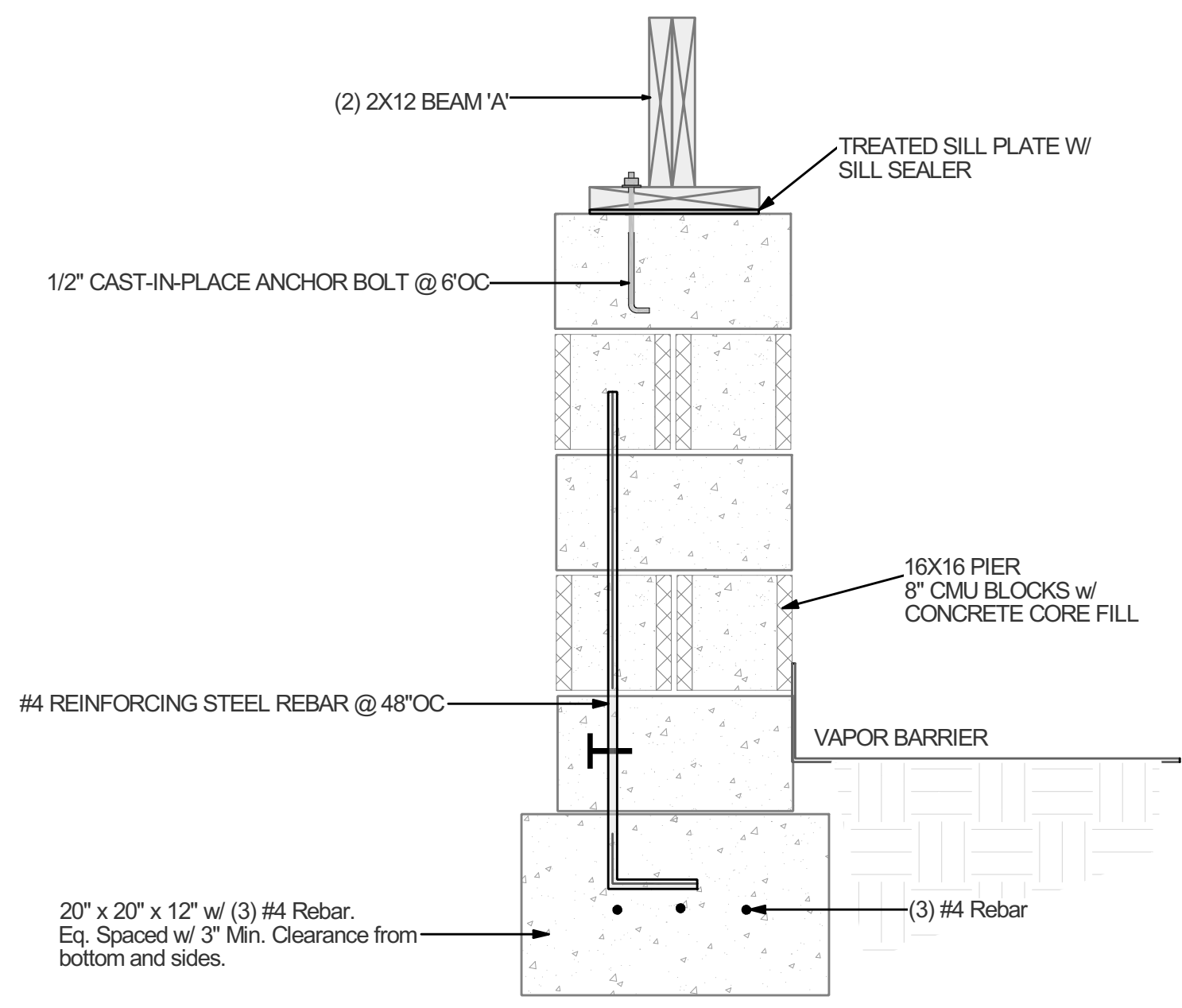
Foundation Plan
Scale: 1/8" = 1'

Required Beam Depths and Bearing Lengths [in] VERSA-LAM 2.0 3100

Load Duration %	Floor Load [psf]		Beam Support Spacing [Feet]	Width of Building Segment [feet]															
	Live	Dead		KEY: Beam Breadth [in] X Beam Depth [in] End Support / Intermediate Support Bearing Length Requirements [in]															
				20	24	26	28	30	32	36	40								
100%	40	10	8	3.5 x 7.25	1.5/3	3.5 x 7.25	1.5/3	3.5 x 9.5	1.5/3	3.5 x 9.5	1.5/3	3.5 x 9.5	1.5/4.5	3.5 x 9.5	1.5/4.5	3.5 x 9.5	3/4.5	3.5 x 9.5	3/4.5
				5.25 x 7.25	1.5/1.5	5.25 x 7.25	1.5/3	5.25 x 7.25	1.5/3	5.25 x 7.25	1.5/3	5.25 x 7.25	1.5/3	5.25 x 7.25	1.5/3	5.25 x 7.25	1.5/3	5.25 x 9.5	1.5/3
			10	3.5 x 9.5	1.5/3	3.5 x 9.5	1.5/4.5	3.5 x 9.5	1.5/4.5	3.5 x 9.5	1.5/4.5	3.5 x 11.875	3/4.5	3.5 x 11.875	3/4.5	3.5 x 11.875	3/6	3.5 x 11.875	3/6
				5.25 x 9.5	1.5/3	5.25 x 9.5	1.5/3	5.25 x 9.5	1.5/3	5.25 x 9.5	1.5/3	5.25 x 9.5	1.5/3	5.25 x 9.5	1.5/3	5.25 x 9.5	1.5/4.5	5.25 x 9.5	1.5/4.5
			12	3.5 x 11.875	1.5/4.5	3.5 x 11.875	3/4.5	3.5 x 11.875	3/4.5	3.5 x 11.875	3/4.5	3.5 x 11.875	3/6	3.5 x 11.875	3/6	3.5 x 14	3/6	3.5 x 14	3/7.5
				5.25 x 9.5	1.5/3	5.25 x 9.5	1.5/3	5.25 x 11.875	1.5/3	5.25 x 11.875	1.5/3	5.25 x 11.875	1.5/3	5.25 x 11.875	1.5/4.5	5.25 x 11.875	1.5/4.5	5.25 x 11.875	3/4.5
	14	3.5 x 11.875	1.5/4.5	3.5 x 14	3/4.5	3.5 x 14	3/6	3.5 x 14	3/6	3.5 x 14	3/6	3.5 x 14	3/6	3.5 x 16	3/7.5	3.5 x 16	3/7.5		
		5.25 x 11.875	1.5/3	5.25 x 11.875	1.5/3	5.25 x 11.875	1.5/4.5	5.25 x 11.875	1.5/4.5	5.25 x 11.875	1.5/4.5	5.25 x 14	3/4.5	5.25 x 14	3/4.5	5.25 x 14	3/6		
	16	3.5 x 14	3/4.5	3.5 x 16	3/6	3.5 x 16	3/6	3.5 x 16	3/6	3.5 x 16	3/6	3.5 x 16	3/7.5	3.5 x 16	3/7.5	3.5 x 18	4.5/9		
		5.25 x 11.875	1.5/3	5.25 x 14	1.5/4.5	5.25 x 14	1.5/4.5	5.25 x 14	1.5/4.5	5.25 x 14	1.5/4.5	5.25 x 14	3/4.5	5.25 x 14	3/4.5	5.25 x 16	3/6		
	18	3.5 x 16	3/6	3.5 x 16	3/6	3.5 x 18	3/7.5	3.5 x 18	3/7.5	3.5 x 18	3/7.5	3.5 x 18	3/7.5	3.5 x 18	4.5/9	5.25 x 16	3/6		
		5.25 x 14	1.5/4.5	5.25 x 14	3/4.5	5.25 x 16	3/4.5	5.25 x 16	3/4.5	5.25 x 16	3/4.5	5.25 x 16	3/6	5.25 x 16	3/6	7 x 16	3/4.5		
20	3.5 x 18	3/6	3.5 x 18	3/7.5	5.25 x 16	3/6	5.25 x 18	3/6	5.25 x 18	3/6	5.25 x 18	3/6	5.25 x 18	3/6	-	-			
	5.25 x 16	1.5/4.5	5.25 x 16	3/4.5	7 x 16	1.5/4.5	7 x 16	1.5/4.5	7 x 16	1.5/4.5	7 x 16	3/4.5	7 x 16	3/4.5	7 x 18	3/6			

TABLE R403.1(1)
MINIMUM WIDTH AND THICKNESS FOR CONCRETE FOOTINGS FOR LIGHT-FRAME CONSTRUCTION (inches)^{a, b}

SNOW LOAD OR ROOF LIVE LOAD	STORY AND TYPE OF STRUCTURE WITH LIGHT FRAME	LOAD-BEARING VALUE OF SOIL (psf)					
		1500	2000	2500	3000	3500	4000
20 psf	1 story—slab-on-grade	12 x 6	12 x 6	12 x 6	12 x 6	12 x 6	12 x 6
	1 story—with crawl space	12 x 6	12 x 6	12 x 6	12 x 6	12 x 6	12 x 6
	1 story—plus basement	18 x 6	14 x 6	12 x 6	12 x 6	12 x 6	12 x 6
	2 story—slab-on-grade	12 x 6	12 x 6	12 x 6	12 x 6	12 x 6	12 x 6
	2 story—with crawl space	16 x 6	12 x 6	12 x 6	12 x 6	12 x 6	12 x 6
	2 story—plus basement	22 x 6	16 x 6	13 x 6	12 x 6	12 x 6	12 x 6
	3 story—slab-on-grade	14 x 6	12 x 6	12 x 6	12 x 6	12 x 6	12 x 6
	3 story—with crawl space	19 x 6	14 x 6	12 x 6	12 x 6	12 x 6	12 x 6
	3 story—plus basement	25 x 8	19 x 6	15 x 6	13 x 6	12 x 6	12 x 6



INTERIOR SUPPORT DETAIL
SCALE: NTS



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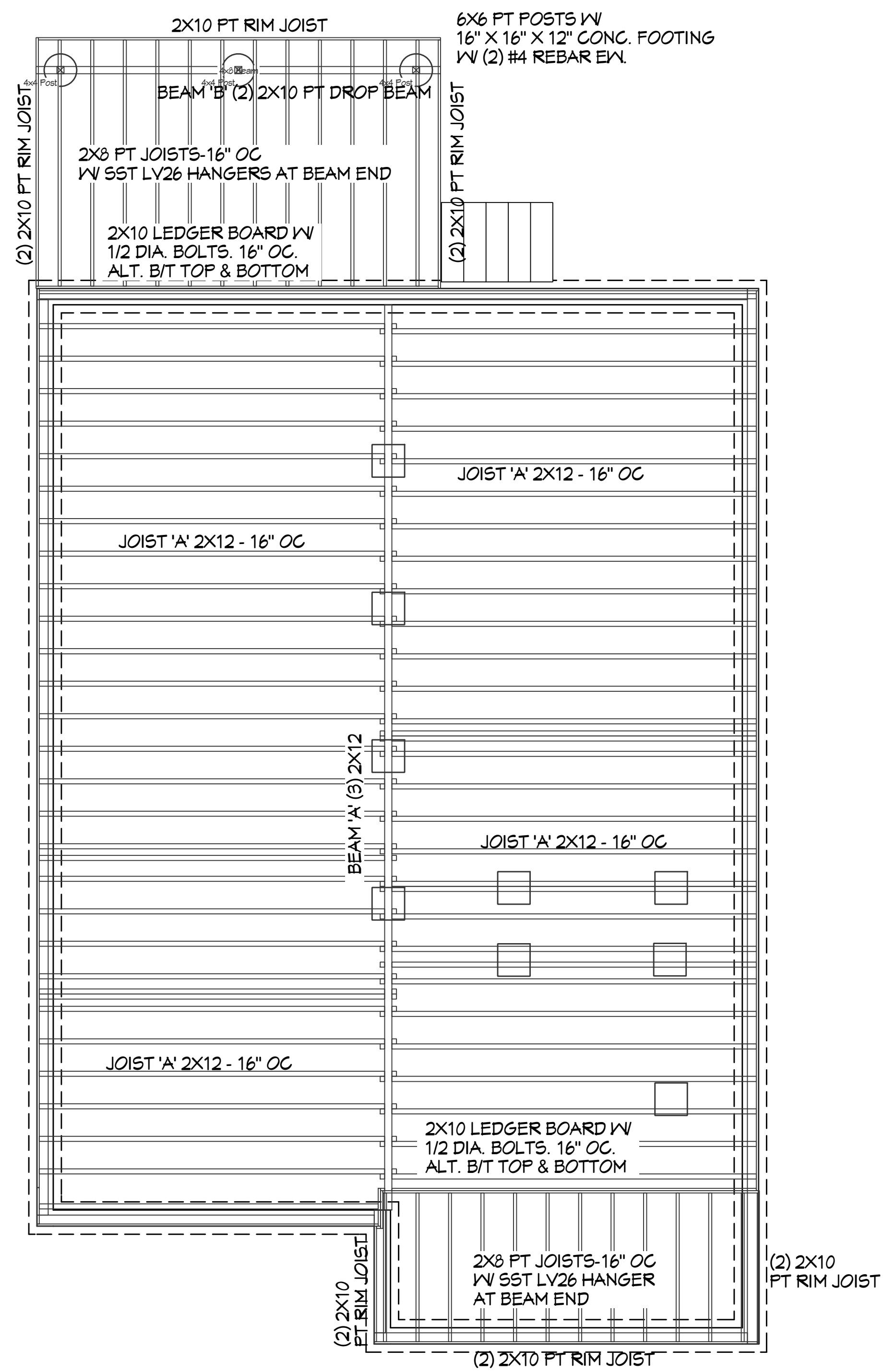
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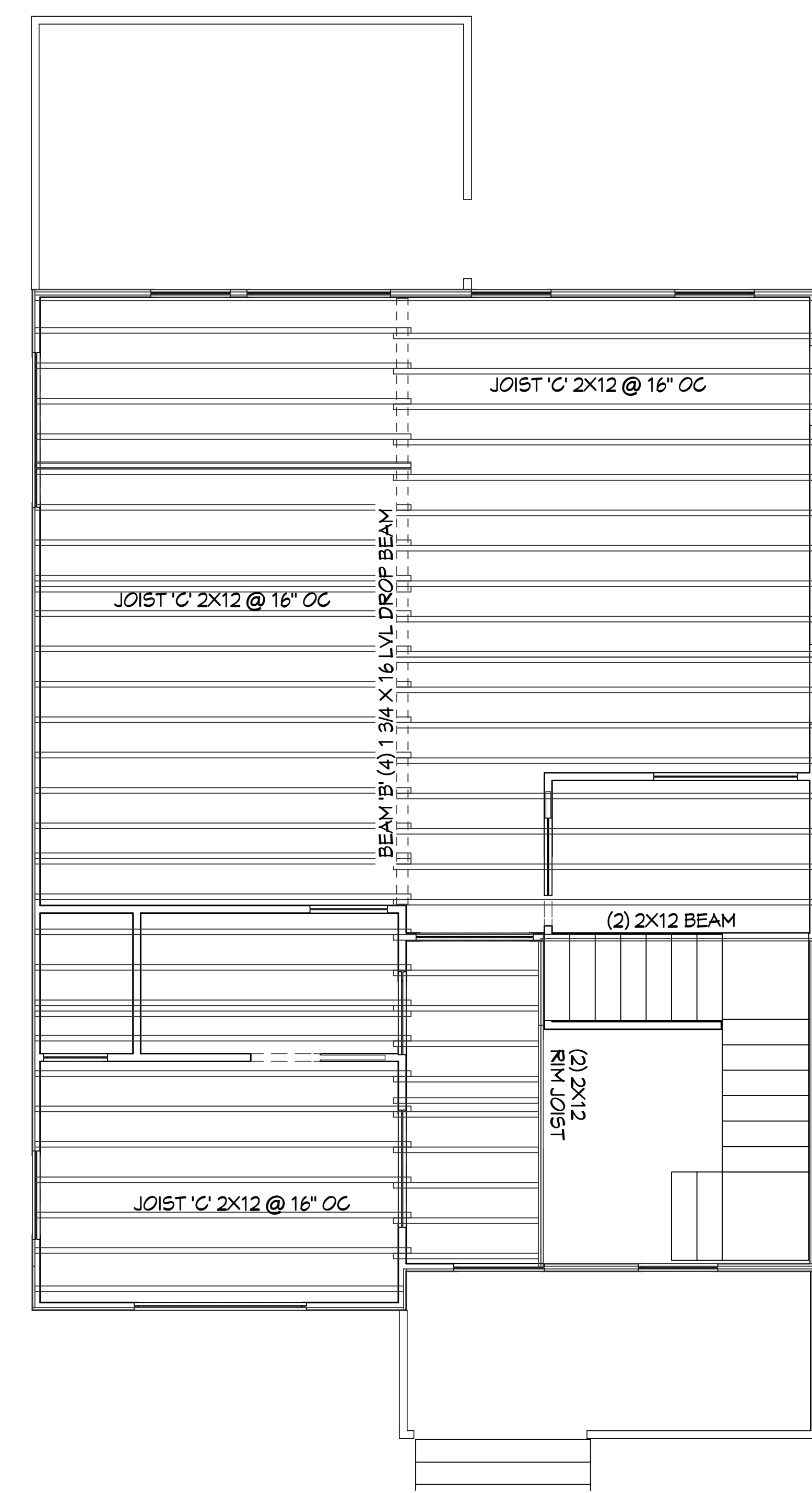
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Floor Framing-First Floor
Scale: 1/8" = 1'



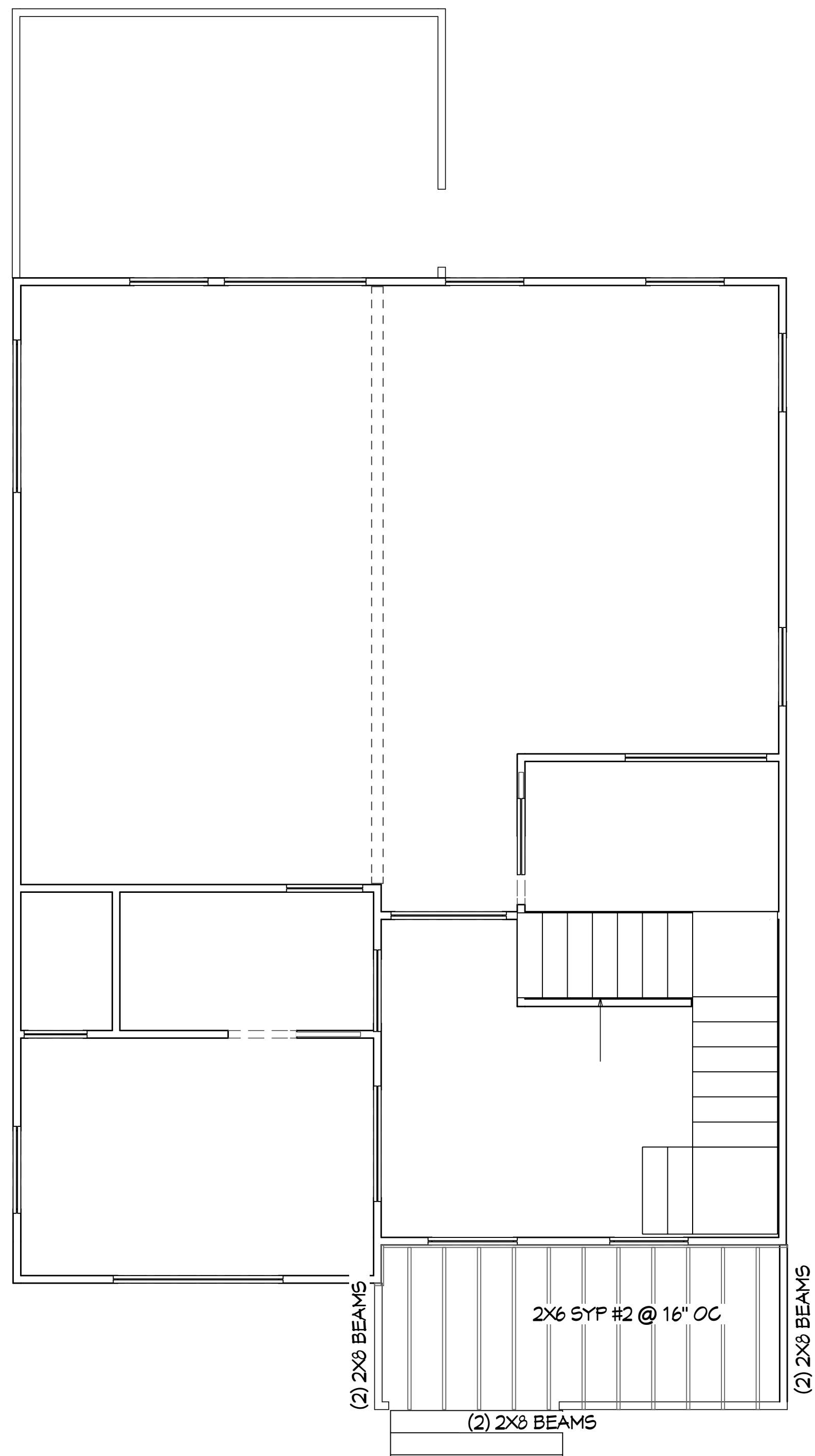
Floor Framing-Second Floor
Scale: 1/8" = 1'

TABLE R502.3.1(2)
FLOOR JOIST SPANS FOR COMMON LUMBER SPECIES (Residential living areas, live load = 40 psf, L/A = 360)^b

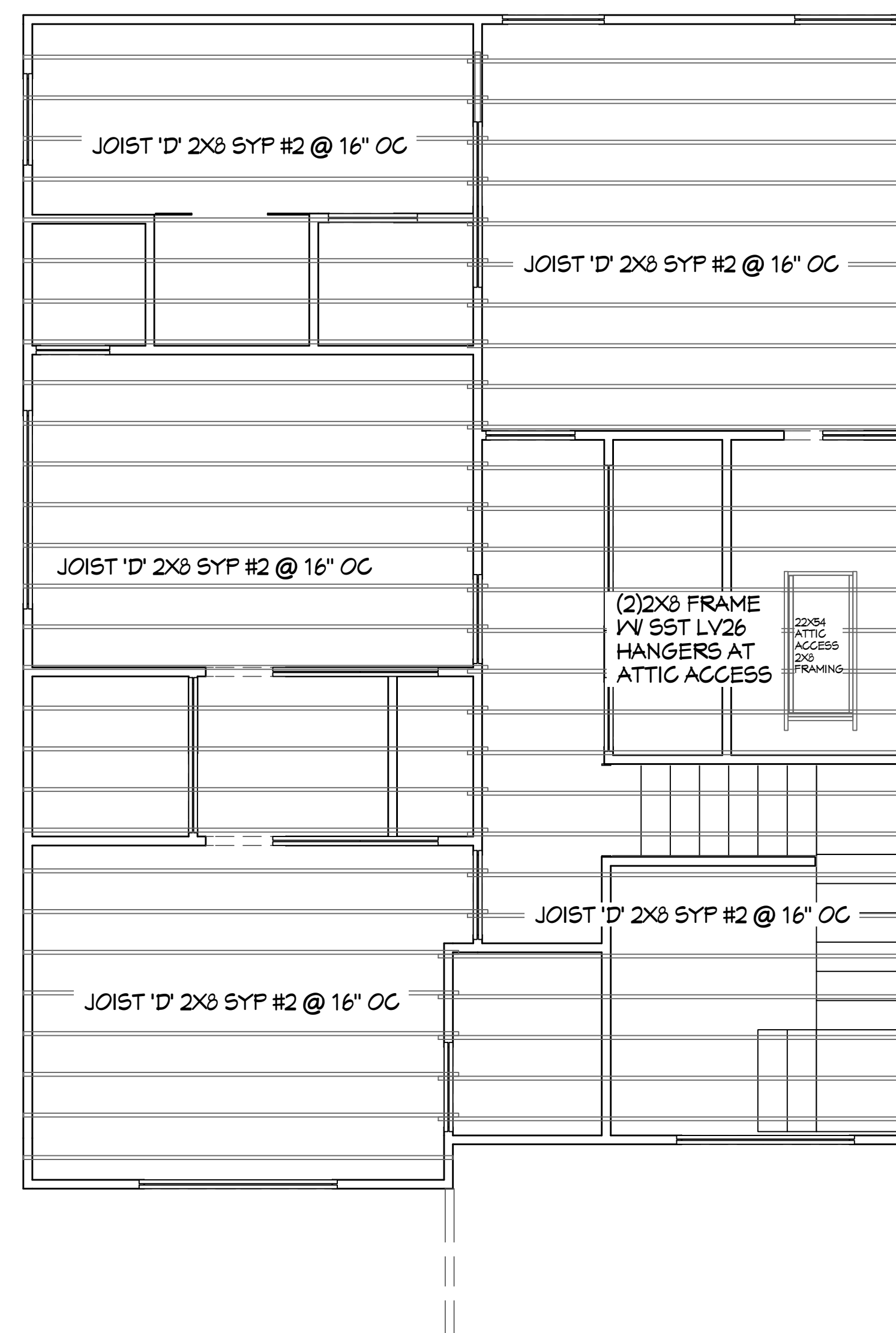
JOIST SPACING (inches)	SPECIES AND GRADE		DEAD LOAD = 10 psf				DEAD LOAD = 20 psf			
			2 x 6	2 x 8	2 x 10	2 x 12	2 x 6	2 x 8	2 x 10	2 x 12
			Maximum floor joist spans							
		(ft. - in.)	(ft. - in.)	(ft. - in.)	(ft. - in.)	(ft. - in.)	(ft. - in.)	(ft. - in.)	(ft. - in.)	
16	Southern pine	SS	10-2	13-4	17-0	20-9	10-2	13-4	17-0	20-9
	Southern pine	#1	9-9	12-10	16-1	19-1	9-9	12-7	14-8	17-5
	Southern pine	#2	9-4	11-10	14-0	16-6	8-6	10-10	12-10	15-1
	Southern pine	#3	7-1	8-11	10-10	12-10	6-5	8-2	9-10	11-8
	Spruce-pine-fir	SS	9-6	12-7	16-0	19-6	9-6	12-7	16-0	19-6
	Spruce-pine-fir	#1	9-4	12-3	15-5	17-10	9-1	11-6	14-1	16-3
	Spruce-pine-fir	#2	9-4	12-3	15-5	17-10	9-1	11-6	14-1	16-3
	Spruce-pine-fir	#3	7-6	9-6	11-8	13-6	6-10	8-8	10-7	12-4

TABLE R502.3.1(1)
FLOOR JOIST SPANS FOR COMMON LUMBER SPECIES (Residential sleeping areas, live load = 30 psf, L/A = 360)^a

JOIST SPACING (inches)	SPECIES AND GRADE		DEAD LOAD = 10 psf				DEAD LOAD = 20 psf			
			2 x 6	2 x 8	2 x 10	2 x 12	2 x 6	2 x 8	2 x 10	2 x 12
			Maximum floor joist spans							
		(ft. - in.)	(ft. - in.)	(ft. - in.)	(ft. - in.)	(ft. - in.)	(ft. - in.)	(ft. - in.)	(ft. - in.)	
12"	Southern pine	SS	12-3	16-2	20-8	25-1	12-3	16-2	20-8	25-1
	Southern pine	#1	11-10	15-7	19-10	24-2	11-10	15-7	18-7	22-0
	Southern pine	#2	11-3	14-11	18-1	21-4	10-9	13-8	16-2	19-1
	Southern pine	#3	9-2	11-6	14-0	16-6	8-2	10-3	12-6	14-9



First Floor-Ceiling Framing
Scale: 1/8" = 1'



Second Floor-Ceiling Framing
Scale: 1/8" = 1'

TABLE R602.7(3)
GIRDER AND HEADER SPANS^a FOR OPEN PORCHES (Maximum span for Douglas fir-larch, hem-fir, Southern pine and spruce-pine-fir^b)

SIZE	SUPPORTING ROOF						SUPPORTING FLOOR	
	GROUND SNOW LOAD (psf)							
	30		50		70			
	DEPTH OF PORCH ^c (feet)							
	8	14	8	14	8	14		
2-2 x 6	7-6	5-8	6-2	4-8	5-4	4-0	6-4	4-9
2-2 x 8	10-1	7-7	8-3	6-2	7-1	5-4	8-5	6-4
2-2 x 10	12-4	9-4	10-1	7-7	8-9	6-7	10-4	7-9
2-2 x 12	14-4	10-10	11-8	8-10	10-1	7-8	11-11	9-0

TABLE R802.5.1(2)
CEILING JOIST SPANS FOR COMMON LUMBER SPECIES (Uninhabitable attics with limited storage, live load = 20 psf, L/Δ = 240)

CEILING JOIST SPACING (inches)	SPECIES AND GRADE	DEAD LOAD = 10 psf				
		2 x 4	2 x 6	2 x 8	2 x 10	
		Maximum ceiling joist spans				
			(feet - inches)	(feet - inches)	(feet - inches)	(feet - inches)
16"	Southern pine	SS	9-4	14-7	19-3	24-7
	Southern pine	#1	8-11	14-0	17-9	20-9
	Southern pine	#2	8-0	12-0	15-3	18-1
	Southern pine	#3	6-2	9-2	11-6	14-0

NUMBER	DATE	REVISION TABLE	REVISOR	DESCRIPTION

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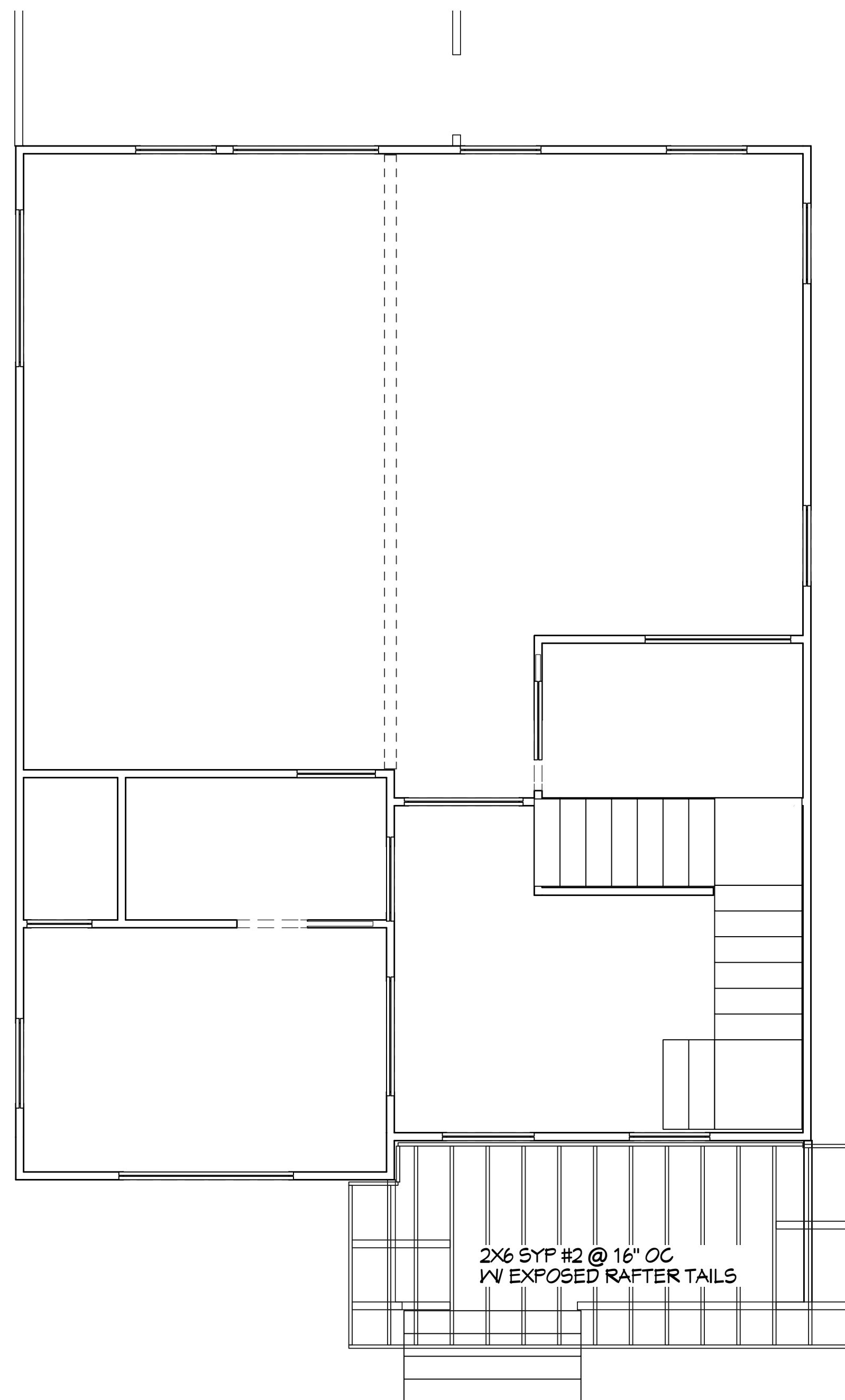


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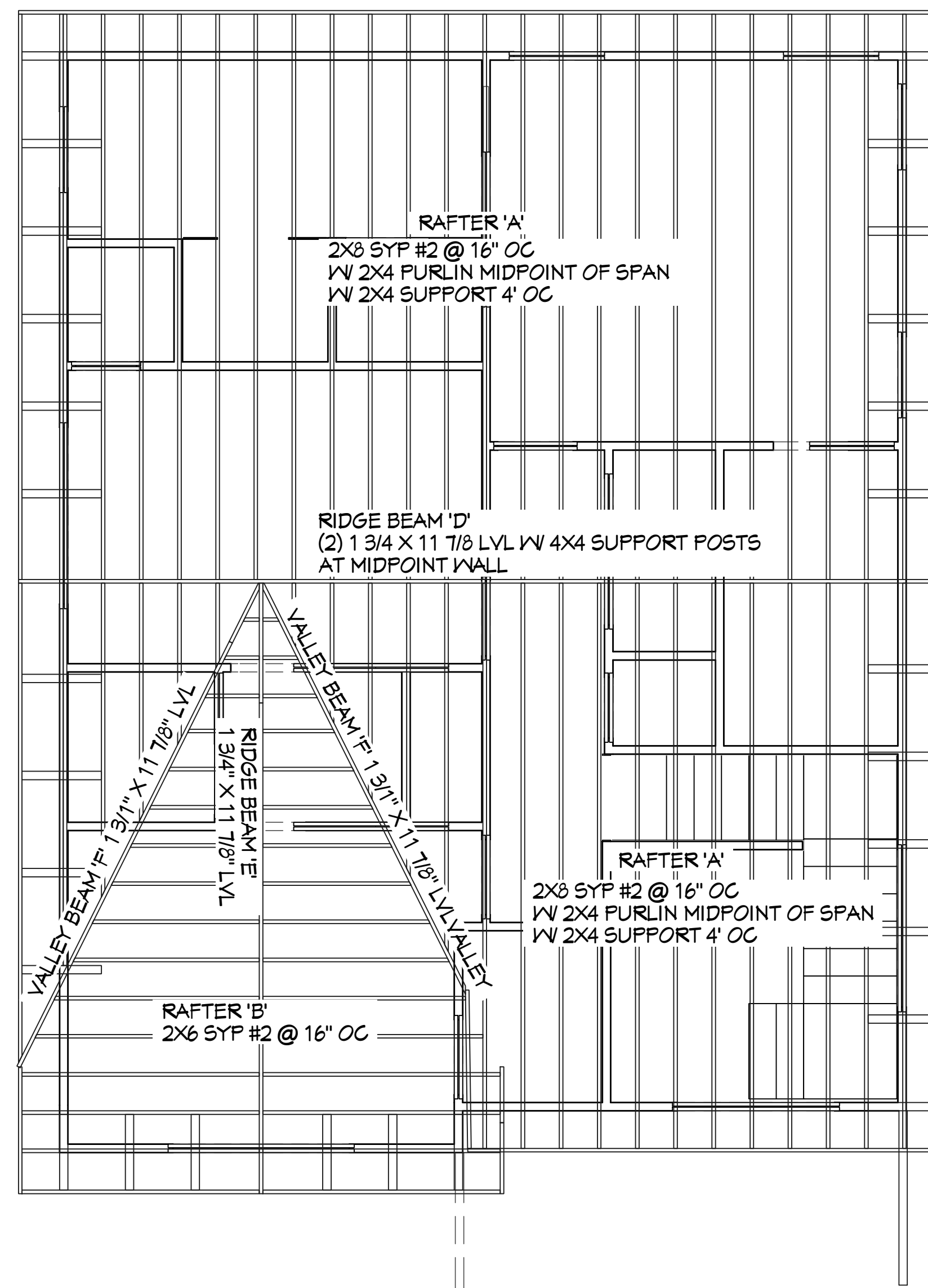
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First Floor-Roof Framing
Scale: 1/8" = 1'

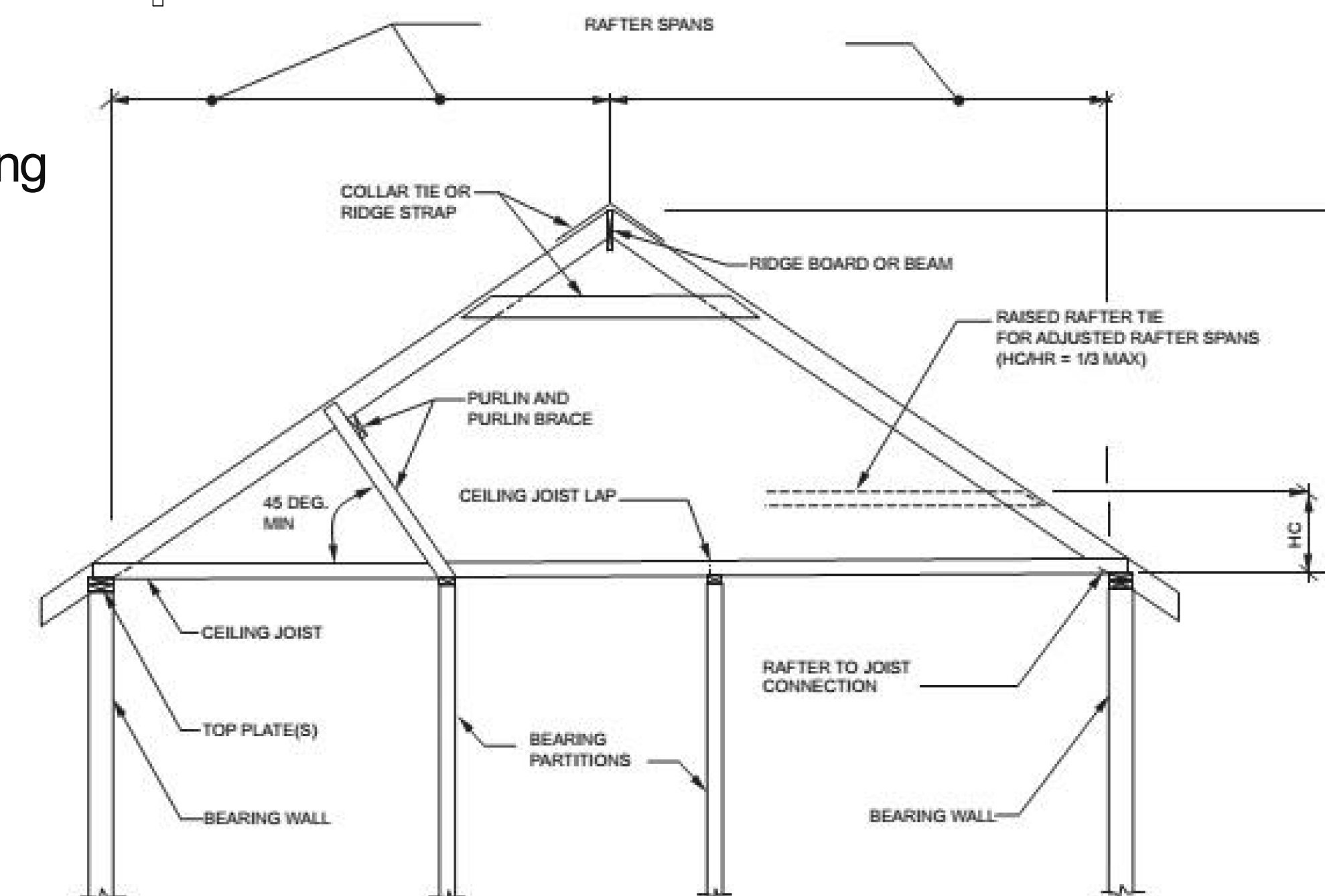


Second Floor-Roof Framing
Scale: 1/8" = 1'

TABLE R802.4.1(3)

RAFTER SPANS FOR COMMON LUMBER SPECIES (Ground snow load = 30 psf, ceiling not attached to rafters, L/A = 180)

RAFTER SPACING (inches)	SPECIES AND GRADE		DEAD LOAD = 10 psf					DEAD LOAD = 20 psf				
			2 x 4	2 x 6	2 x 8	2 x 10	2 x 12	2 x 4	2 x 6	2 x 8	2 x 10	2 x 12
			Maximum rafter spans ^a									
		(feet - inches)	(feet - inches)	(feet - inches)	(feet - inches)	(feet - inches)	(feet - inches)	(feet - inches)	(feet - inches)	(feet - inches)	(feet - inches)	
16"	Southern pine	SS	8-11	14-1	18-6	23-8	Note b	8-11	14-1	18-5	1-11	25-11
	Southern pine	#1	8-7	13-0	16-6	19-3	22-10	7-10	11-7	14-9	17-3	20-5
	Southern pine	#2	7-6	11-2	14-2	16-10	19-10	6-8	10-0	12-8	15-1	17-9
	Southern pine	#3	5-9	8-6	10-8	13-0	15-4	5-2	7-7	9-7	11-7	13-9



For SI: 1 inch = 25.4 mm, 1 foot = 305 mm, 1 degree = 0.018 rad.

H_C = Height of ceiling joists or rafter ties measured vertically above the top of rafter support
 H_R = Height of roof ridge measured vertically above the top of the rafter support walls.

FIGURE R802.4.5
BRACED RAFTER CONSTRUCTION



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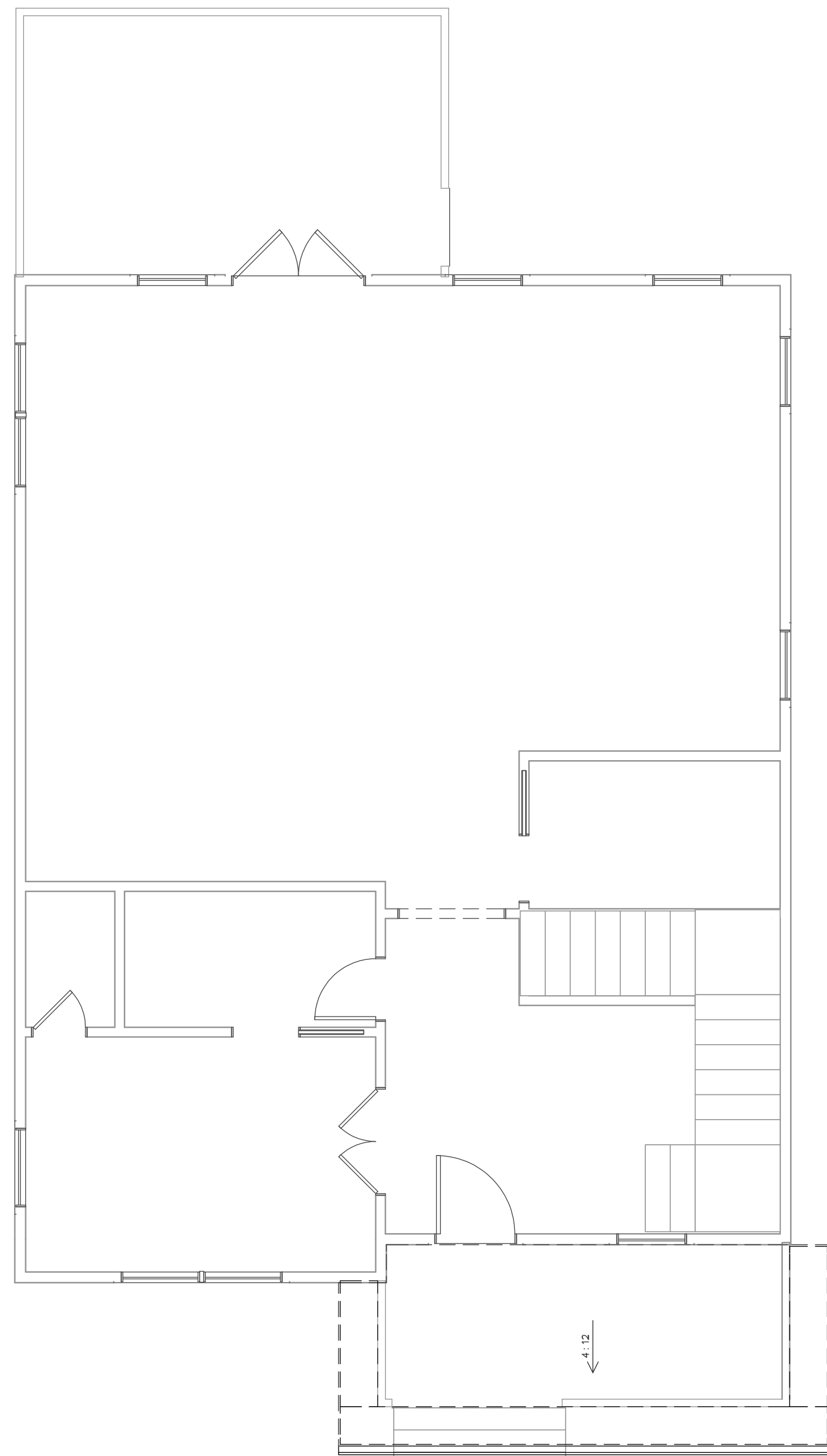


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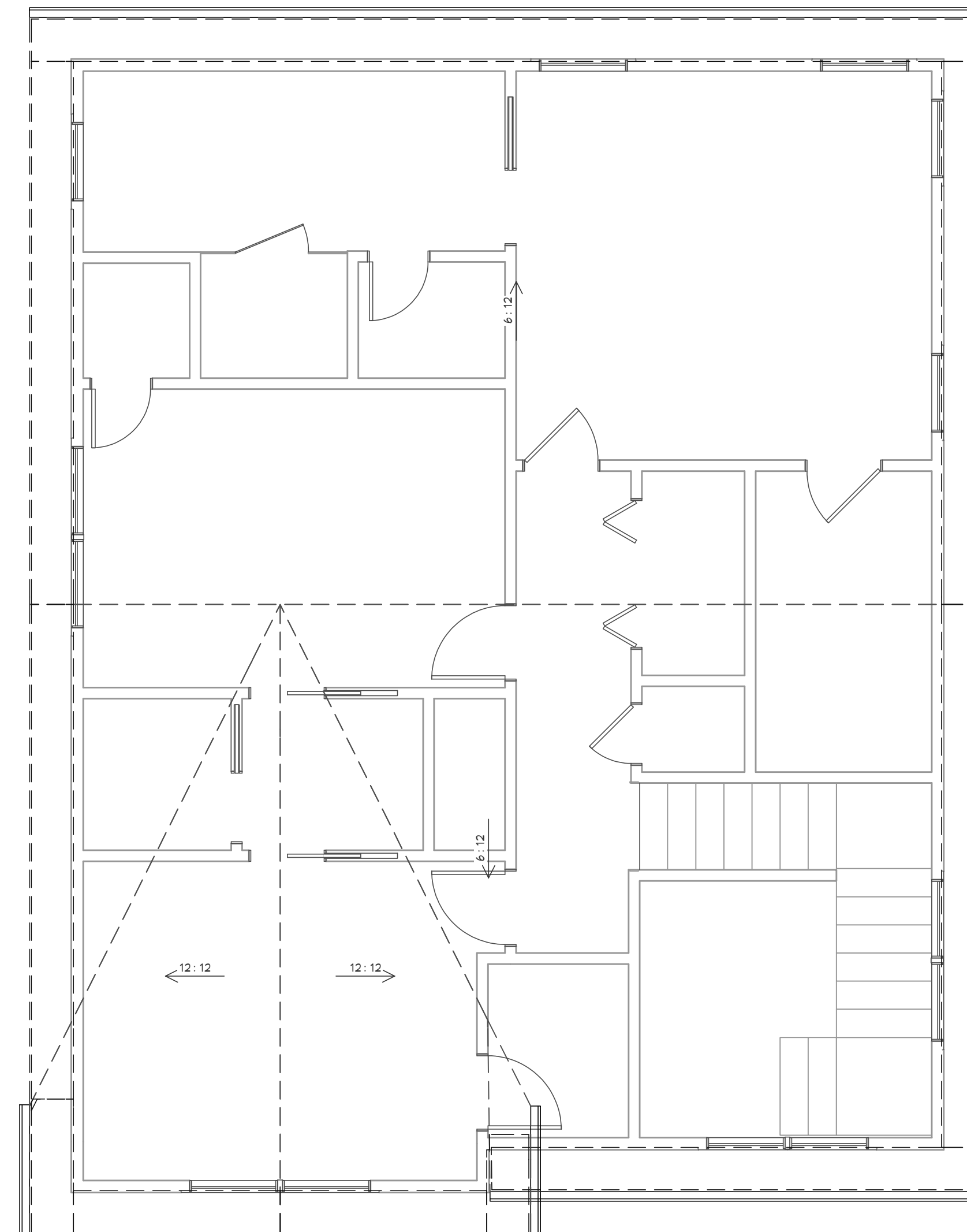
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First Floor-Roof Plan
Scale: 1/8" = 1'



Second Floor-Roof Plan
Scale: 1/8" = 1'



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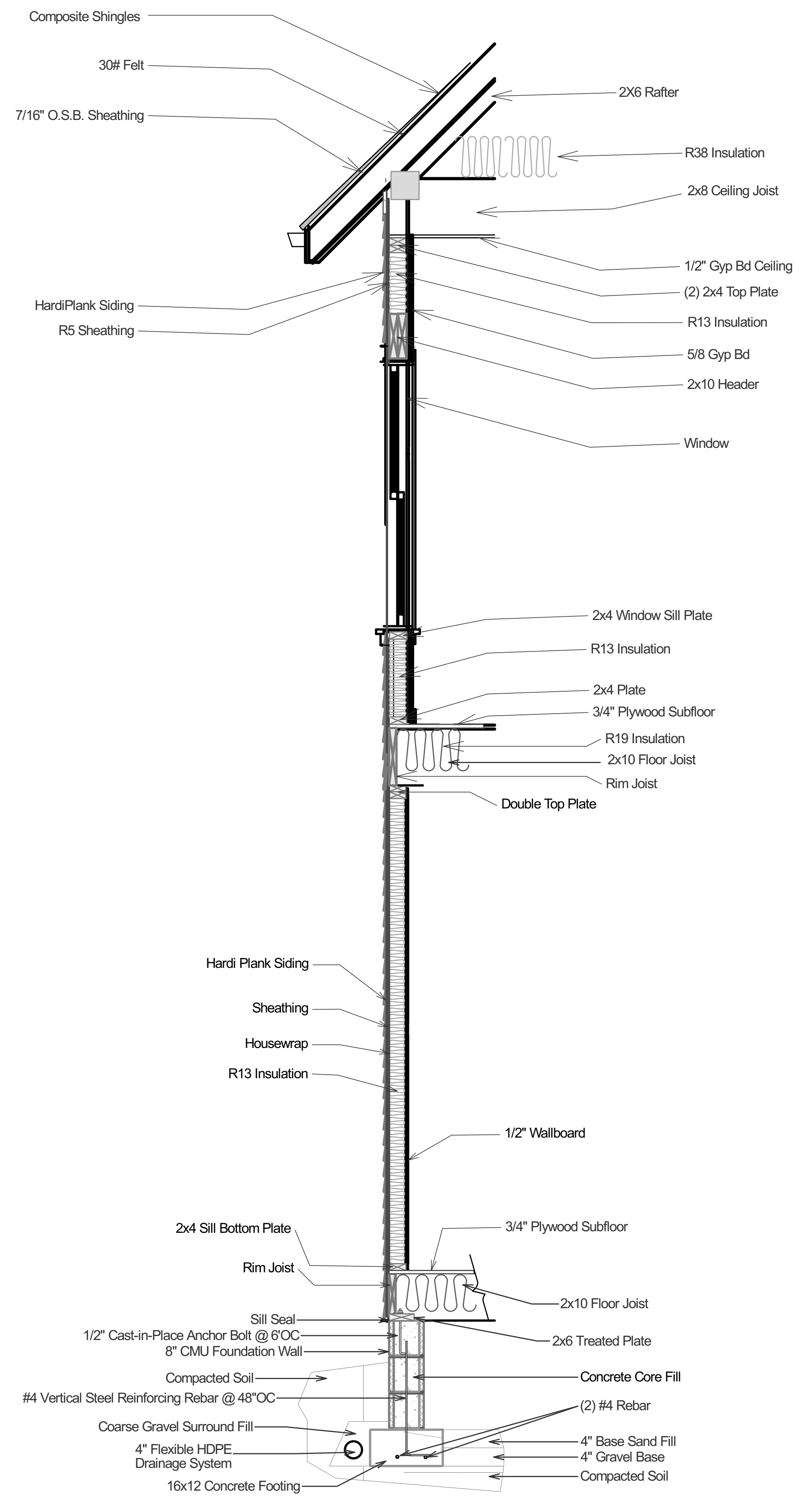
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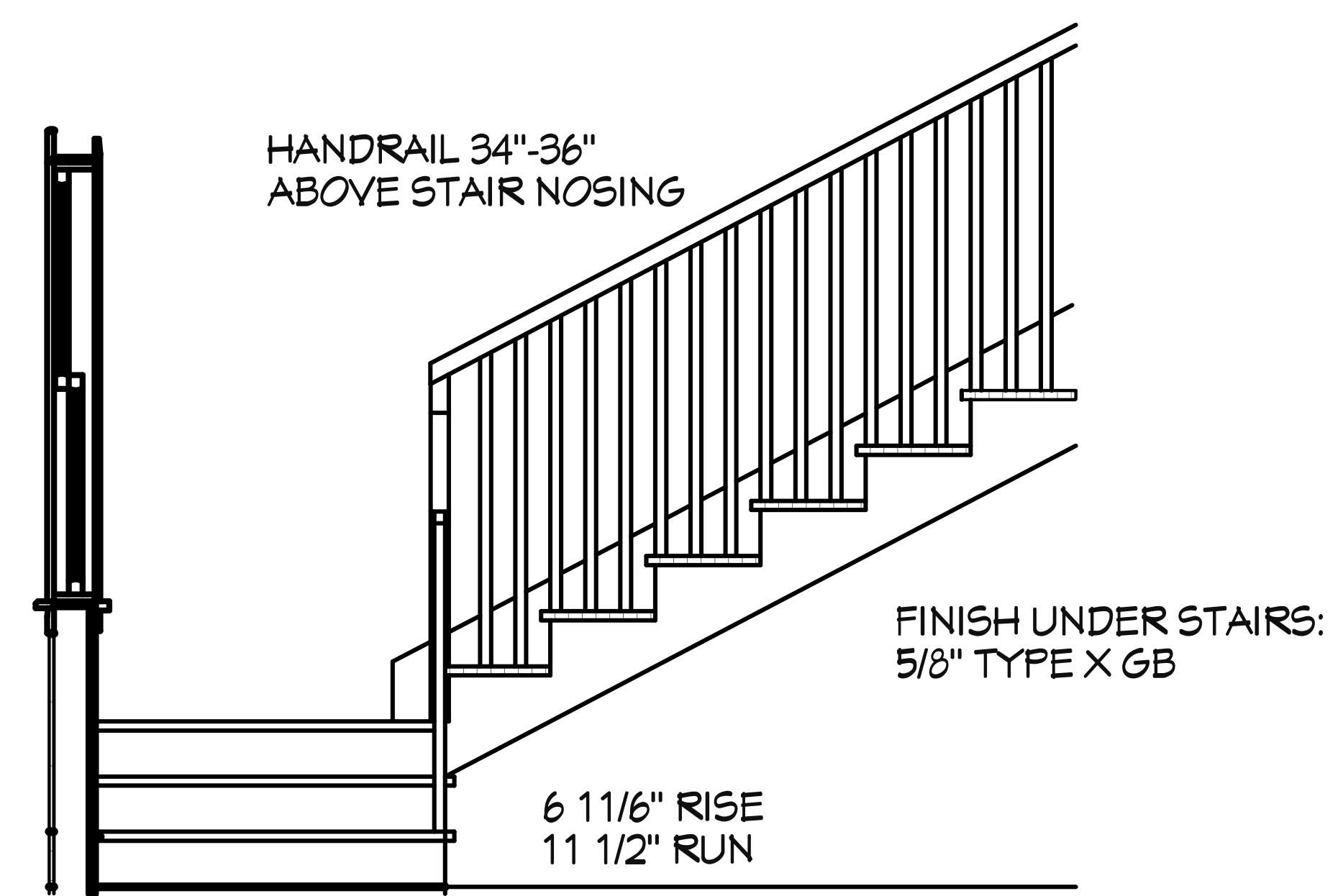
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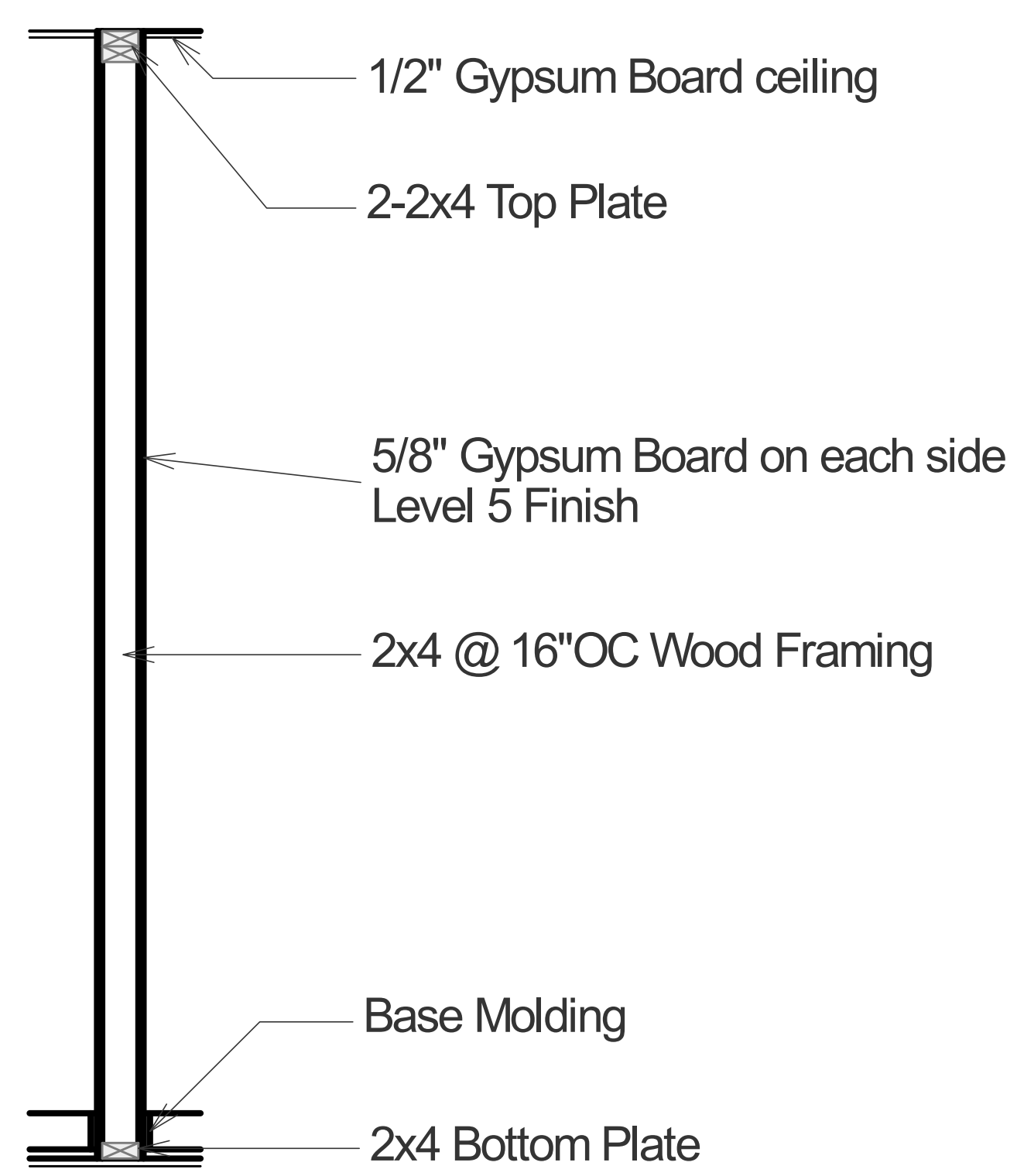
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TYP. EXTERIOR WALL
SCALE: NTS



Interior Stair Detail
Scale: 3/8" = 1'



Interior Wall Detail

Table 5. Fastener Spacing for a Southern Pine, Douglas Fir-Larch, or Hem-Fir Deck Ledger or Band or Rim Joist and a 2-inch Nominal Solid-Sawn Spruce-Pine-Fir Band Joist or LVL Rim Joist.^{3,4,5,6,8}
(Deck Live Load = 40 psf, Deck Dead Load = 10 psf)

Connection Details	Rim Joist or Band Joist	Joist Span						
		6'-0" and less	6'-1" to 8'-0"	8'-1" to 10'-0"	10'-1" to 12'-0"	12'-1" to 14'-0"	14'-1" to 16'-0"	16'-1" to 18'-0"
On-Center Spacing of Fasteners								
1/2" diameter lag screw ¹ with 15/32" maximum sheathing	1" LVL	24"	18"	14"	12"	10"	9"	8"
	1-1/8" LVL 1-1/2" Lumber	28" 30"	21" 23"	16" 18"	14" 15"	12" 13"	10" 11"	9" 10"
1/2" diameter bolt with 15/32" maximum sheathing	1" LVL	24"	18"	14"	12"	10"	9"	8"
	1-1/8" LVL 1-1/2" Lumber	28" 36"	21" 36"	16" 34"	14" 29"	12" 24"	10" 21"	9" 19"
1/2" diameter bolt with 15/32" maximum sheathing and 1/2" stacked washers ^{2,7}	1-1/2" Lumber	36"	36"	29"	24"	21"	18"	16"

- The tip of the lag screw shall fully extend beyond the inside face of the band or rim joist.
- The maximum gap between the face of the ledger board and face of the wall sheathing shall be 1/2".
- Ledgers shall be flashed or caulked to prevent water from contacting the house band joist (see Figures 14 and 15).
- Lag screws and bolts shall be staggered per Figure 19.
- Deck ledgers shall be minimum 2x8 pressure-preservative-treated No.2 grade lumber, or other approved materials as established by standard engineering practice.
- When solid-sawn pressure-preservative-treated deck ledgers are attached to engineered wood products (minimum 1" thick wood structural panel band joist or structural composite lumber including laminated veneer lumber), the ledger attachment shall be designed in accordance with accepted engineering practice. Tabulated values based on 300 lbs and 350 lbs for 1" and 1-1/8" LVL rim joist, respectively.
- Wood structural panel sheathing, gypsum board sheathing, or foam sheathing shall be permitted between the band or rim joist and ledger. Stacked washers are permitted in combination with wood structural panel sheathing, but are not permitted in combination with gypsum board or foam sheathing. The maximum distance between the face of the ledger board and the face of the band joist shall be 1".
- Fastener spacing also applies to Southern Pine, Douglas Fir-Larch, and Hem-Fir band or rim joists.

Figure 19. Ledger Board Fastener Spacing and Clearances.

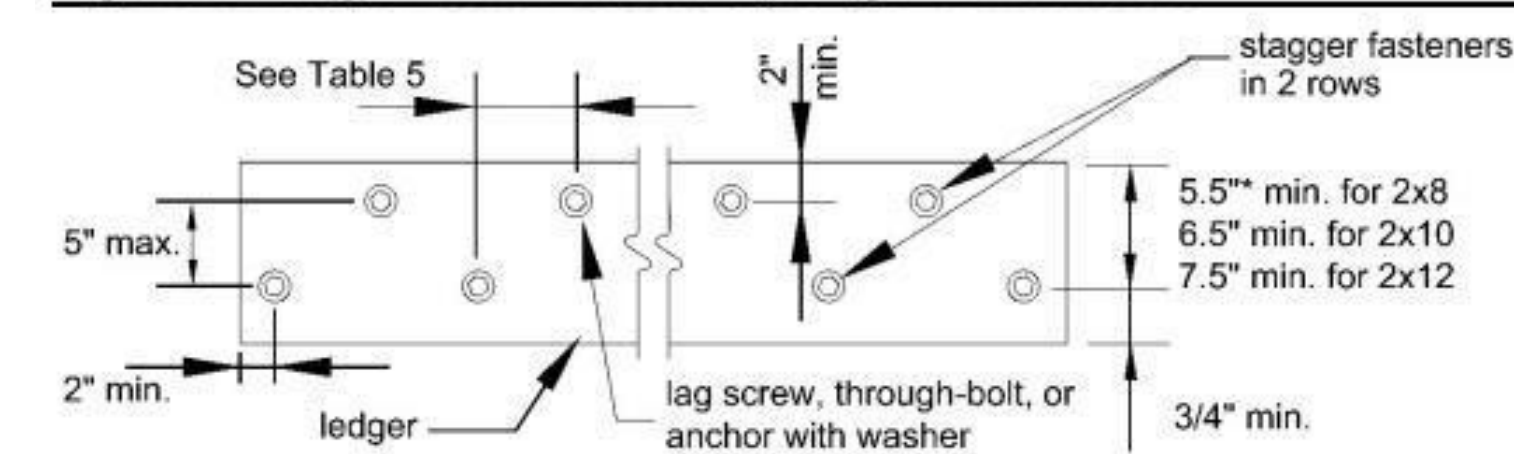


Figure 14. General Attachment of Ledger Board to Band Joist or Rim Joist.

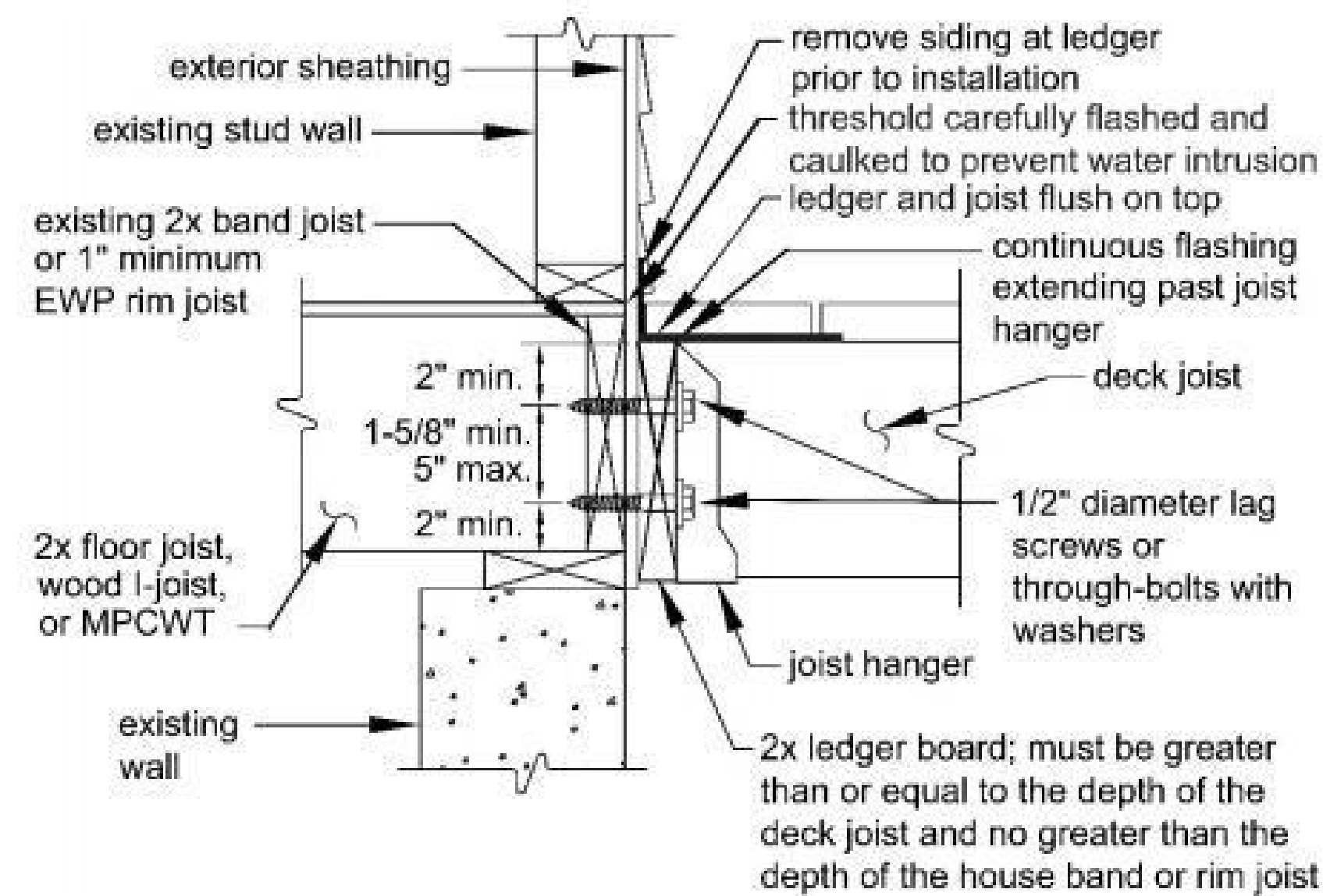


Figure 15. Attachment of Ledger Board to Foundation Wall (Concrete or Solid Masonry).

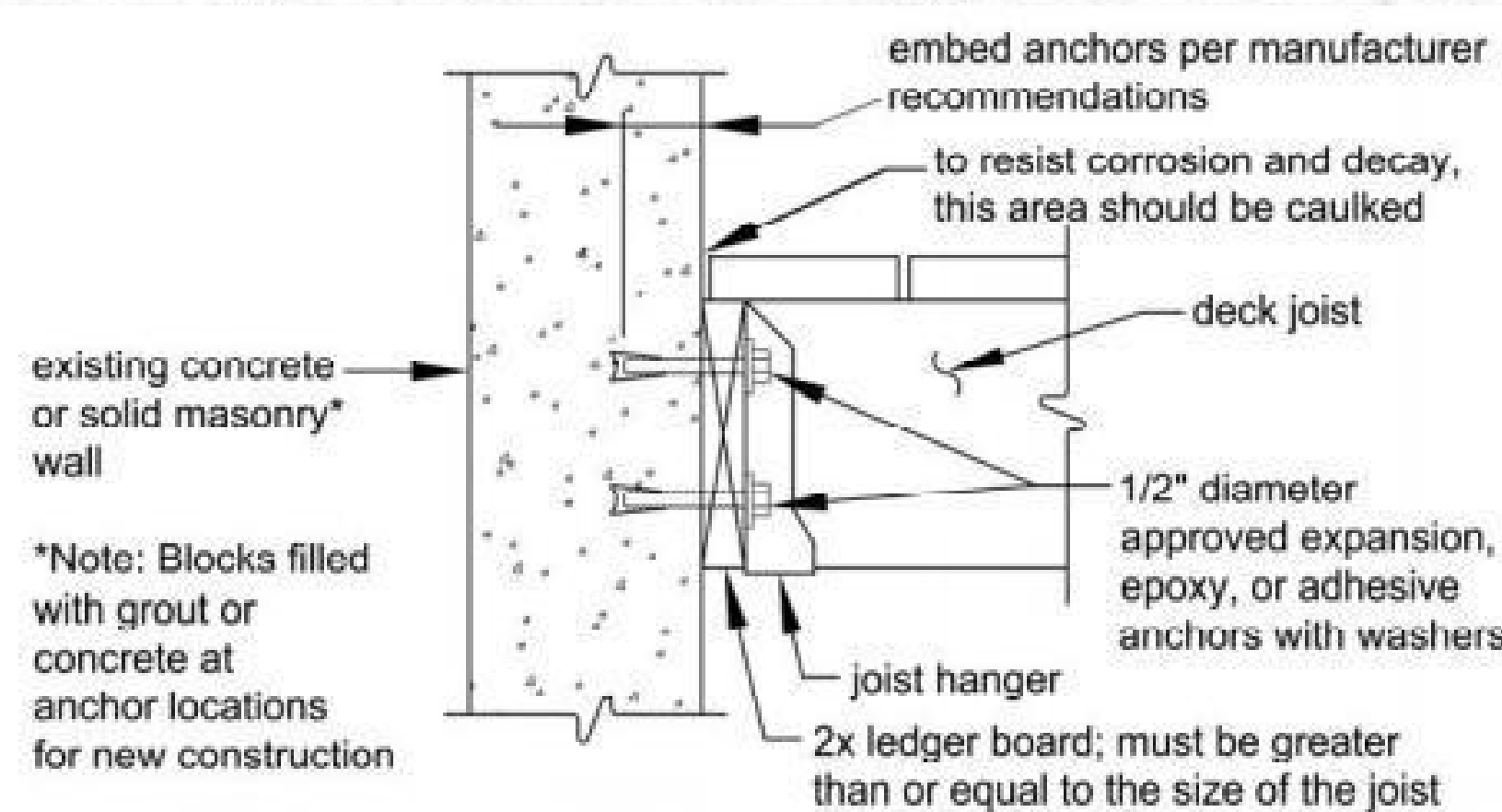
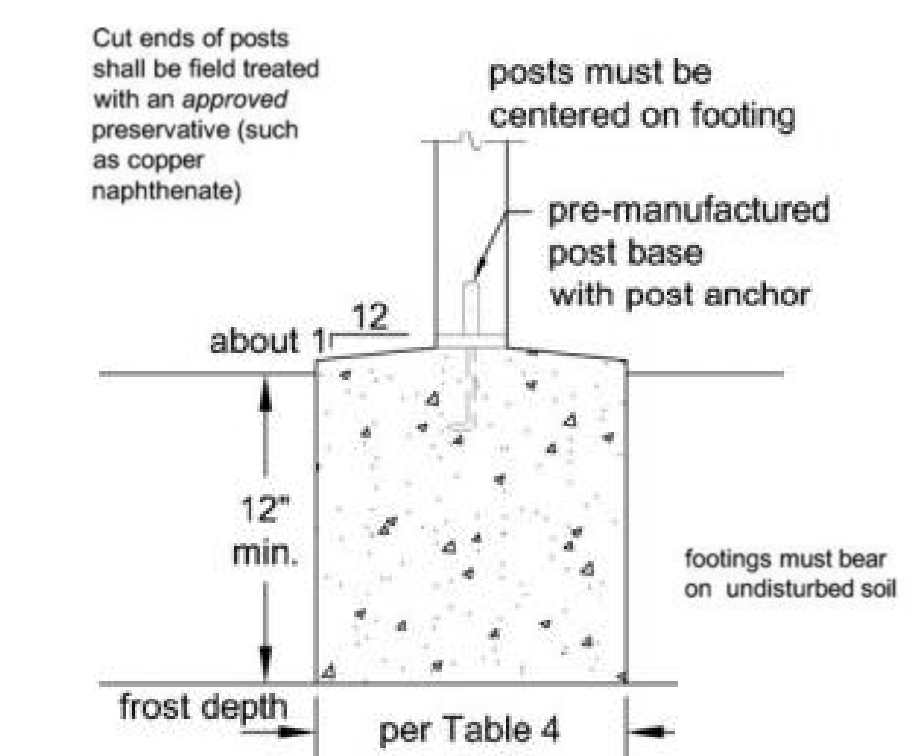


Table 4. Post Height for 6x6⁵ and Footing Sizes for all Posts.

Beam Span, L _b	Joist Span L _j	Post Heights ¹					Footing Sizes ²		
		Southern Pine	Douglas Fir-Larch ³	Hem-Fir, Western Cedars	Redwood	Ponderosa Pine, Red Pine, SPP ³	Round Footing Diameter	Square Footing	Footing Thickness ⁴
6'	≤10'	14'	14'	14'	14'	14'	18"	16"x16"	7"
	≤14'	14'	14'	14'	14'	14'	21"	18"x18"	8"
	≤18'	14'	14'	12'	14'	11'	24"	21"x21"	10"
8'	≤10'	14'	14'	14'	14'	14'	20"	18"x18"	8"
	≤14'	14'	14'	14'	14'	11'	24"	21"x21"	10"
	≤18'	14'	13'	11'	12'	8'	27"	24"x24"	11"
10'	≤10'	14'	14'	14'	14'	12'	23"	20"x20"	9"
	≤14'	14'	13'	11'	13'	8'	27"	24"x24"	11"
	≤18'	12'	11'	8'	11'	2'	31"	27"x27"	13"
12'	≤10'	14'	14'	12'	14'	10'	25"	22"x22"	10"
	≤14'	13'	12'	9'	11'	5'	30"	26"x26"	13"
	≤18'	11'	9'	6'	9'	2'	34"	30"x30"	15"
14'	≤10'	14'	13'	11'	13'	8'	27"	24"x24"	11"
	≤14'	11'	10'	7'	10'	2'	32"	29"x29"	14"
	≤18'	9'	8'	2'	8'	NP	37"	33"x33"	16"
16'	≤10'	13'	12'	10'	12'	6'	29"	26"x26"	12"
	≤14'	10'	9'	5'	9'	2'	35"	31"x31"	15"
	≤18'	7'	5'	2'	7'	NP	40"	35"x35"	18"
18'	≤10'	12'	11'	8'	11'	2'	31"	27"x27"	13"
	≤14'	9'	8'	2'	8'	NP	37"	33"x33"	16"
	≤18'	5'	2'	2'	6'	NP	42"	37"x37"	19"

- Assumes 40 psf live load, 10 psf dead load, L_b/4 and L_j/4 overhangs, No 2. grade and wet service conditions.
- Assumes 1,500 psf soil bearing capacity and 150 pcf concrete. Value may be multiplied by 0.9 for corner posts.
- Incising assumed for Douglas Fir-Larch, Hem-Fir, and Spruce-Pine-Fir.
- Assumes 2,500 psi compressive strength of concrete. Coordinate footing thickness with post base and anchor requirements.
- 8x8 nominal posts may be substituted anywhere in Table 4 to a maximum height of 14'.

Figure 12. Typical Footing



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D1



Table 3A. Dimension Lumber Deck Beam Spans (L_b) Supporting a Single Span of Joists with or without Overhangs.

Species	Size ^d	Joist Spans (L) Less Than or Equal to:						
		6'	8'	10'	12'	14'	16'	18'
Southern Pine	2-2x6	6'-8"	5'-8"	5'-1"	4'-7"	4'-3"	4'-0"	3'-9"
	2-2x8	8'-6"	7'-4"	6'-6"	5'-11"	5'-6"	5'-1"	4'-9"
	2-2x10	10'-1"	8'-9"	7'-9"	7'-1"	6'-6"	6'-1"	5'-9"
	2-2x12	11'-11"	10'-4"	9'-2"	8'-4"	7'-9"	7'-3"	6'-9"
	3-2x6	7'-11"	7'-2"	6'-5"	5'-10"	5'-5"	5'-0"	4'-9"
	3-2x8	10'-7"	9'-3"	8'-3"	7'-6"	6'-11"	6'-5"	6'-1"
	3-2x10	12'-9"	11'-0"	9'-9"	8'-9"	8'-3"	7'-8"	7'-3"
	3-2x12	15'-0"	13'-0"	11'-7"	10'-6"	9'-9"	9'-1"	8'-7"

Table 2. Maximum Joist Spans and Overhangs.¹

Species	Size	Joist Spacing (o.c.)			Allowable Span ² (L _j)	Allowable Overhang ³ (L _o)
		12"	16"	24"		
Southern Pine	2x6 ⁶	9'-11"	9'-0"	7'-7"	1'-0"	1'-1"
	2x8	13'-1"	11'-10"	9'-8"	1'-10"	2'-0"
	2x10	16'-2"	14'-0"	11'-5"	3'-1"	3'-5"
	2x12	18'-0" ⁷	16'-6"	13'-6"	4'-6"	4'-2"

Figure 1A. Joist Span – Joists Attached at House and Bearing Over Beam.

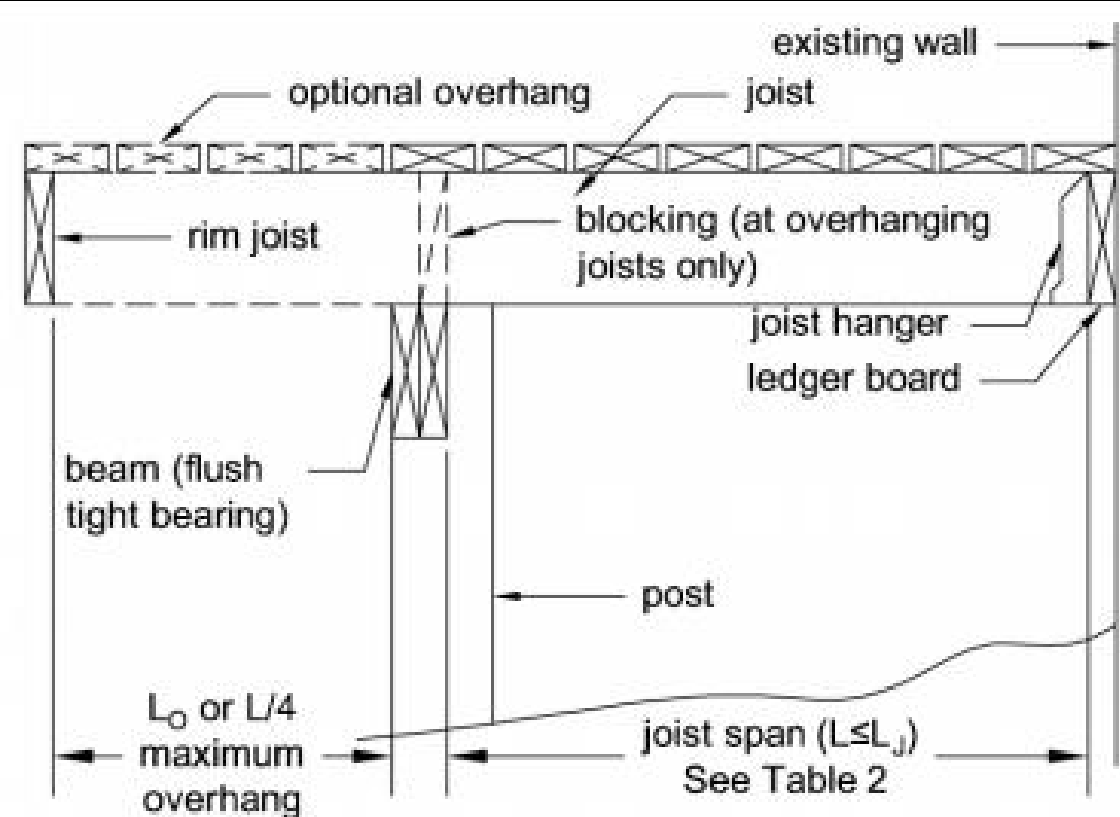


Figure 8A. Post-to-Beam Attachment Requirements.

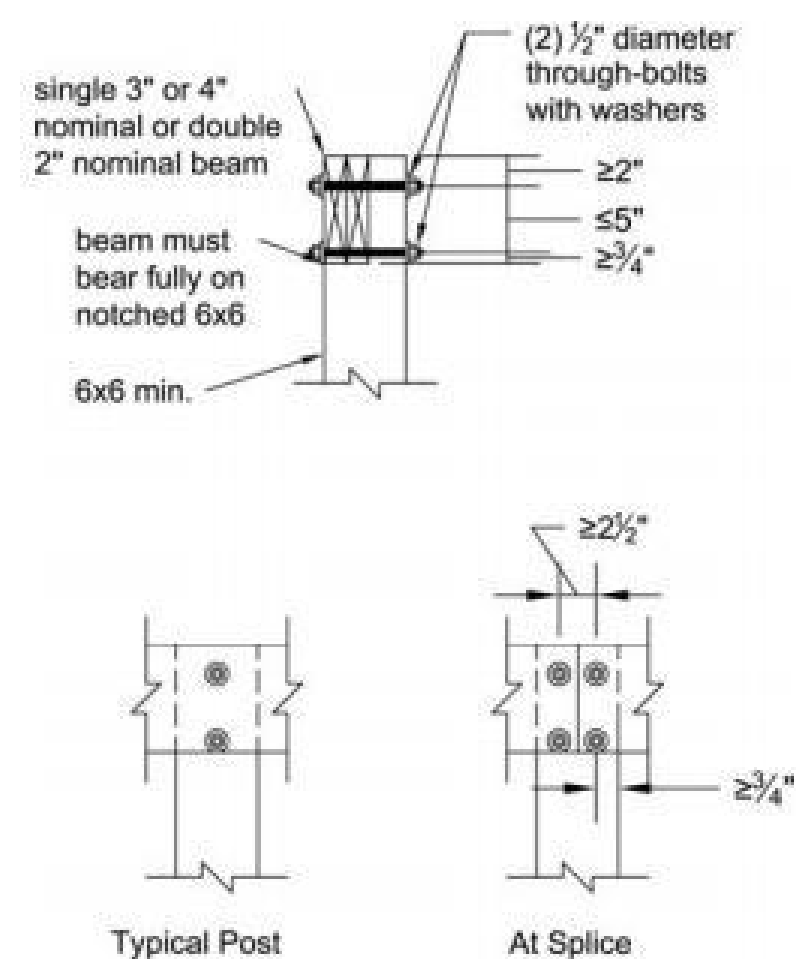


Figure 10. Diagonal Bracing.

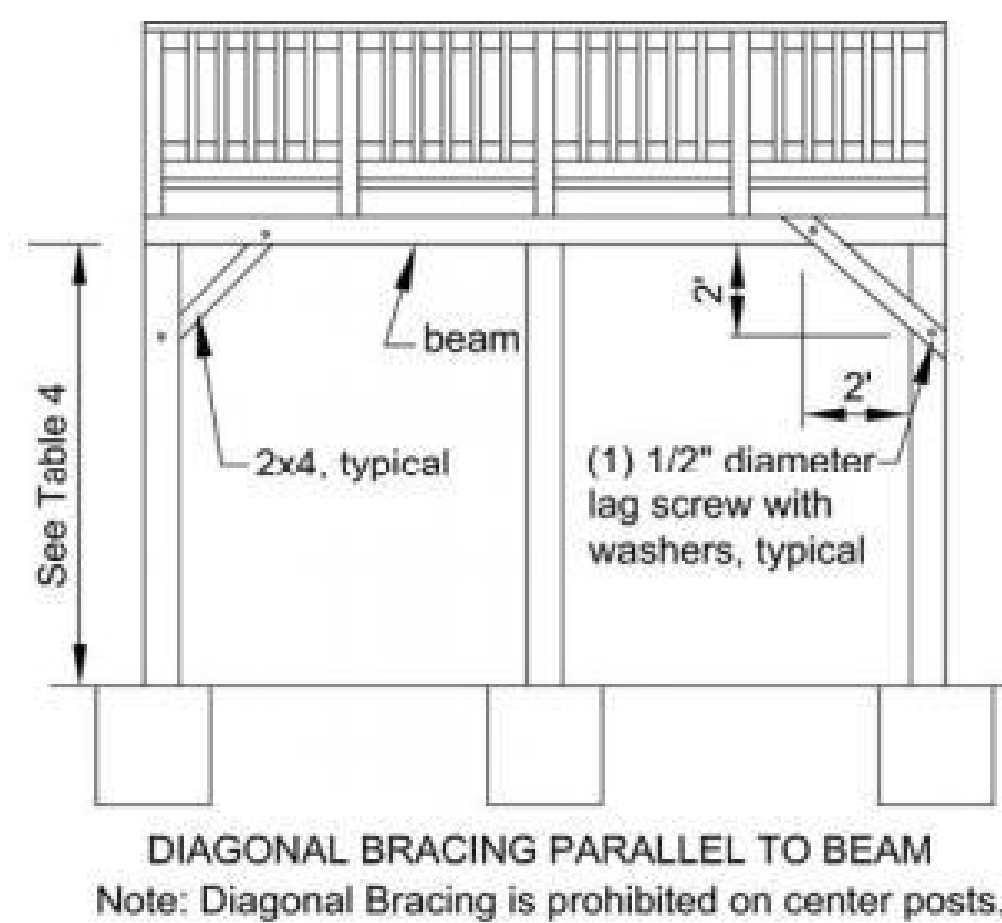


Figure 24. Example Guard Detail.

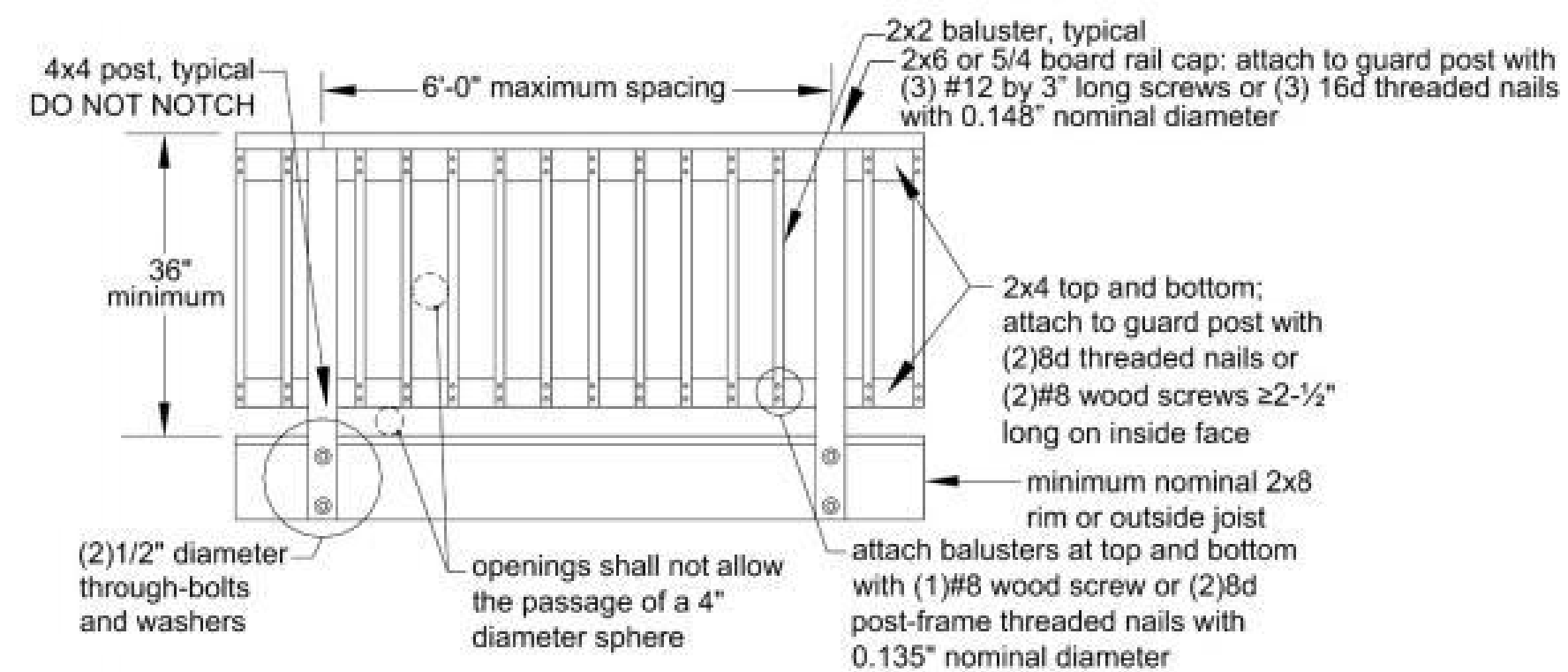


Figure 26. Guard Post to Rim Joist Example.

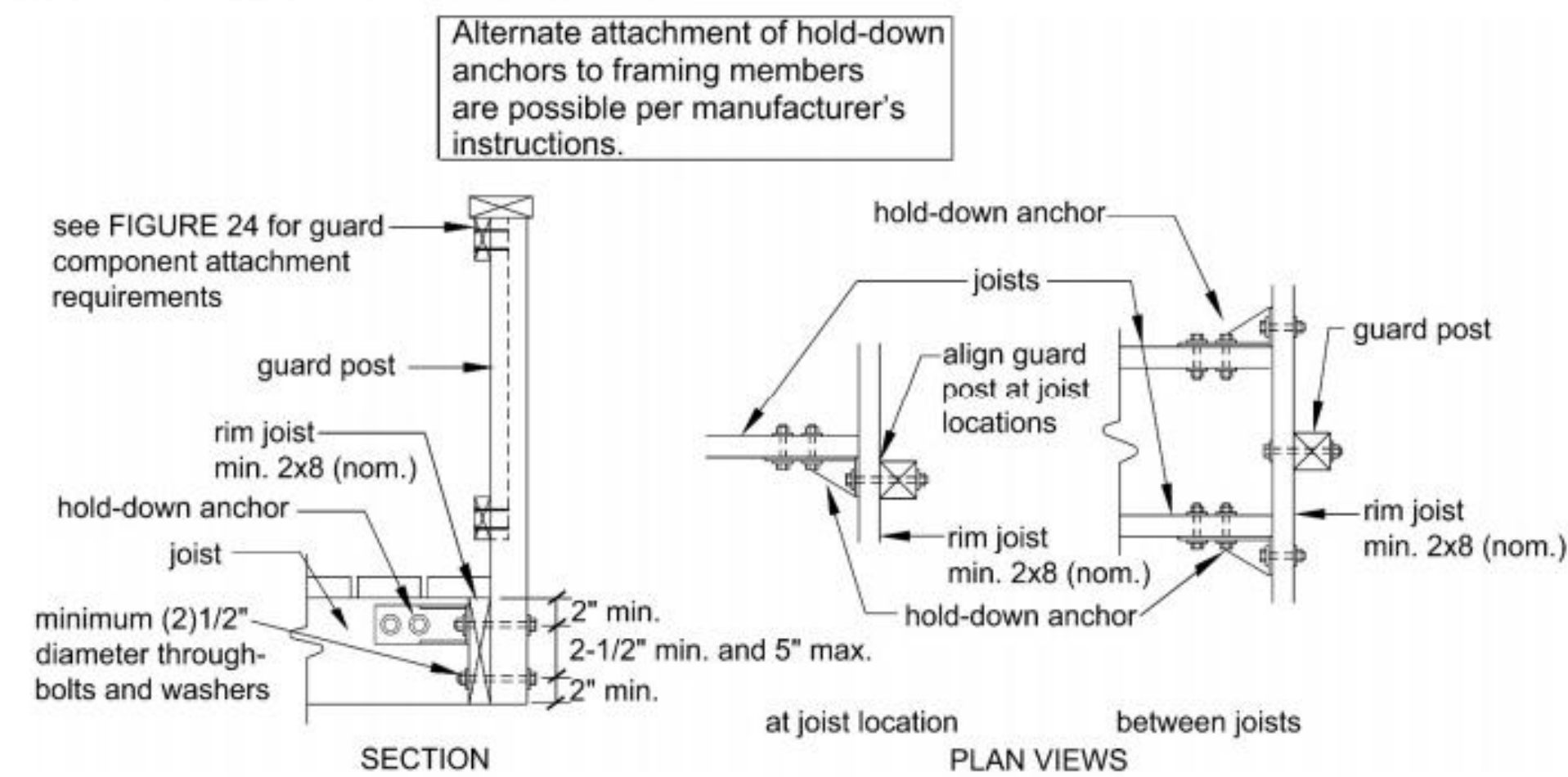


Figure 6. Joist-to-Beam Detail.

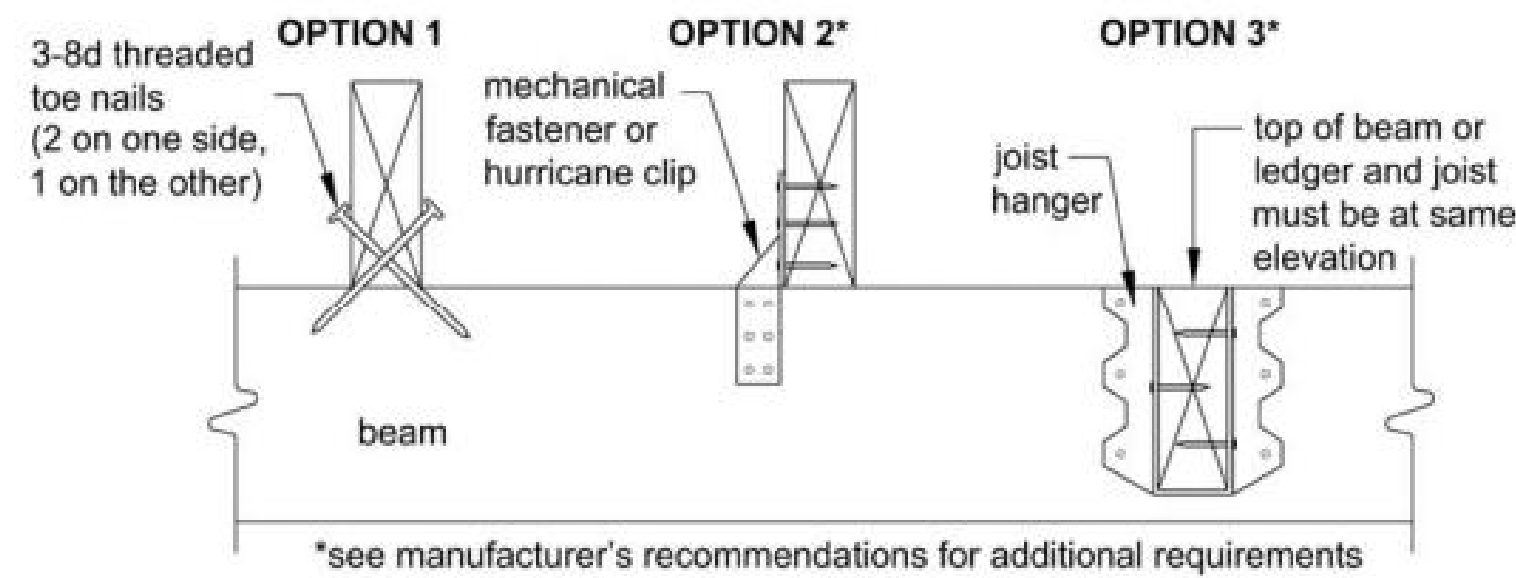


Table 3A. Joist Hanger Vertical Capacity.

Joist Size	Minimum Capacity, lbs
2x6	400
2x8	500
2x10	600
2x12	700

Figure 34. Stair Footing Detail.

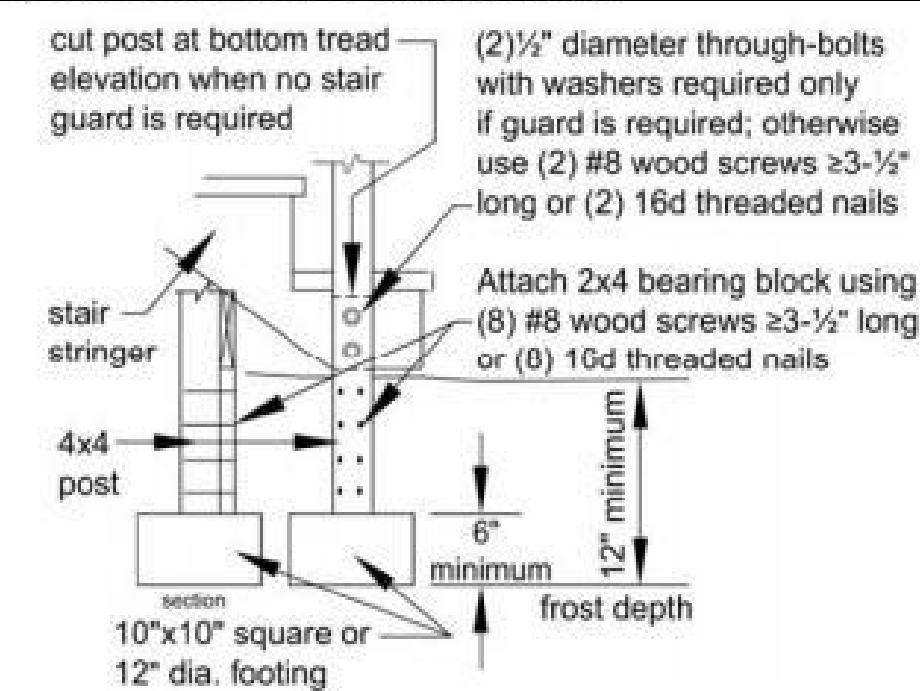


Figure 27. Tread and Riser Detail.

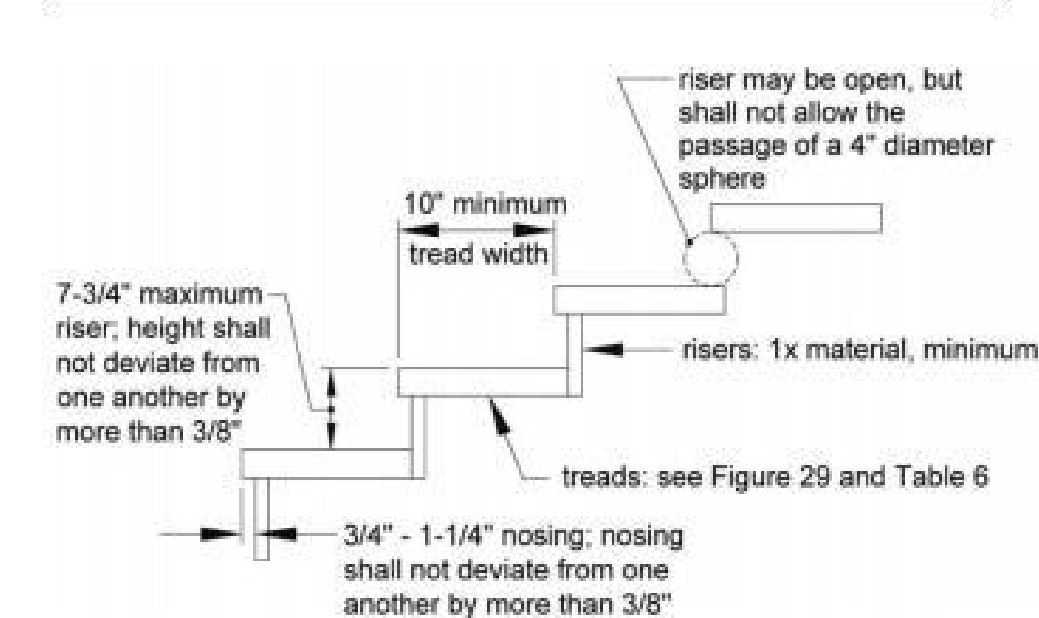


Figure 30. Stair Guard Requirements.

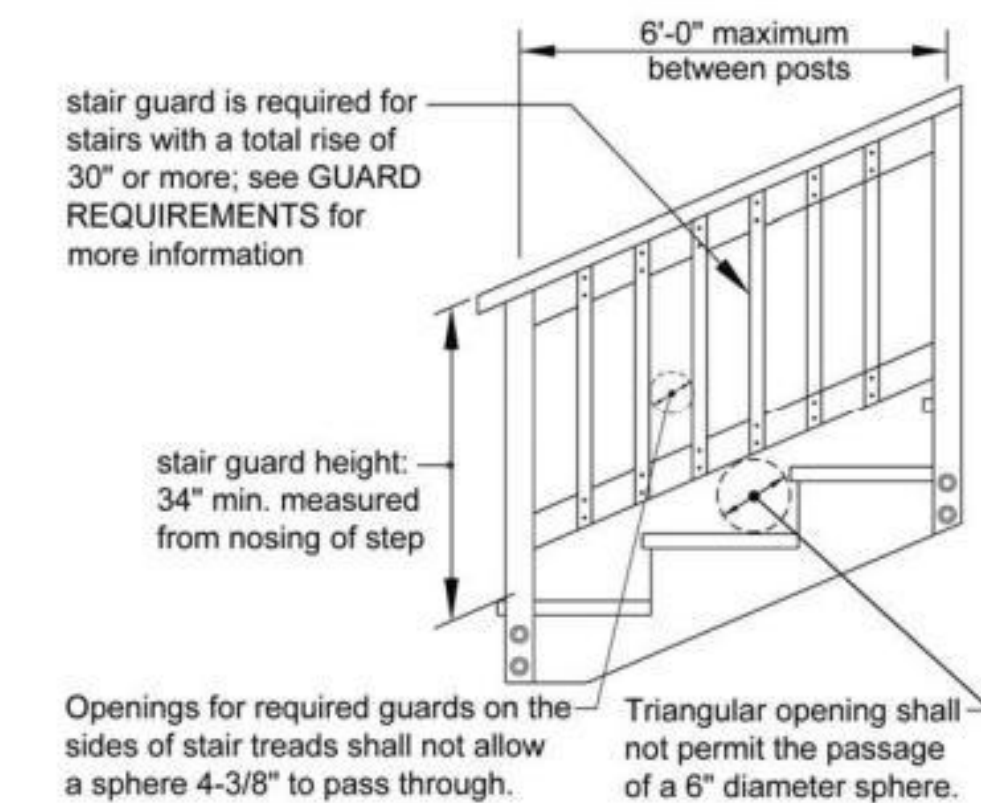
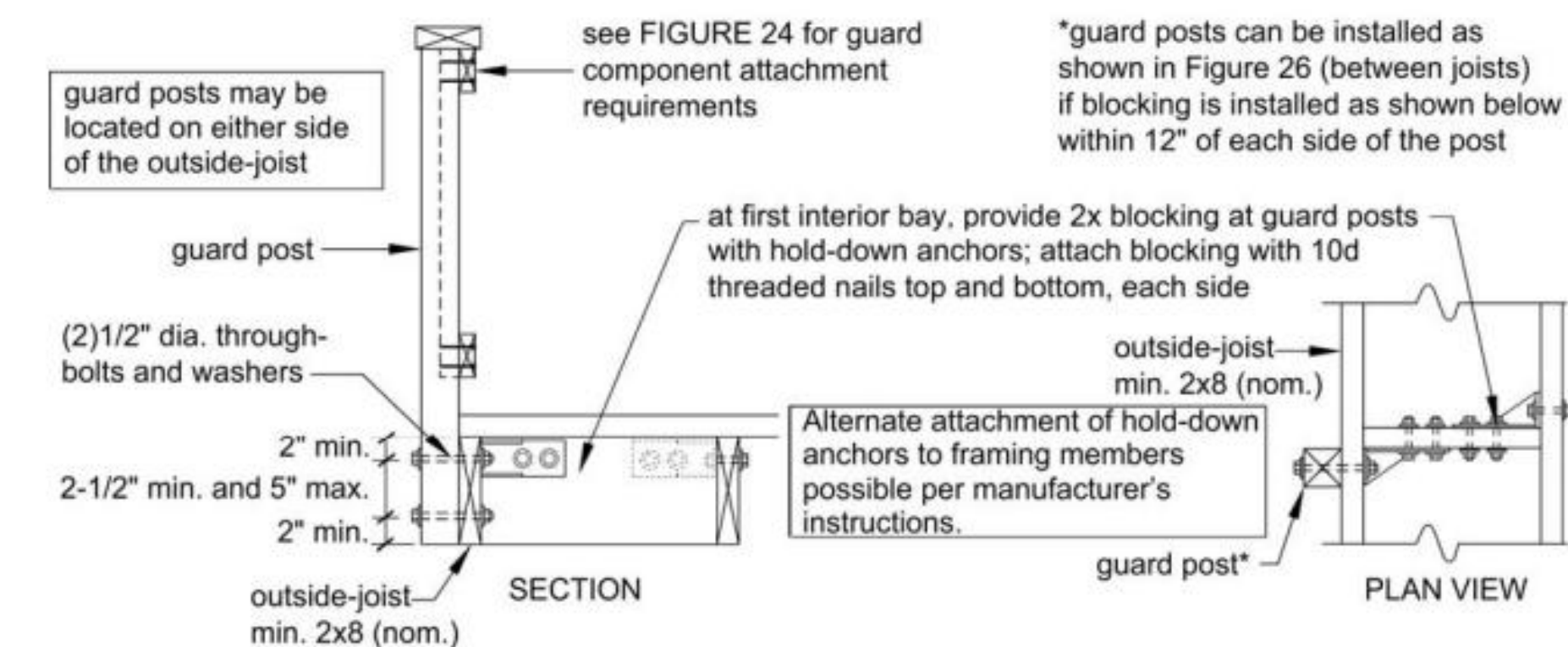


Figure 25. Guard Post to Outside-Joist Example.



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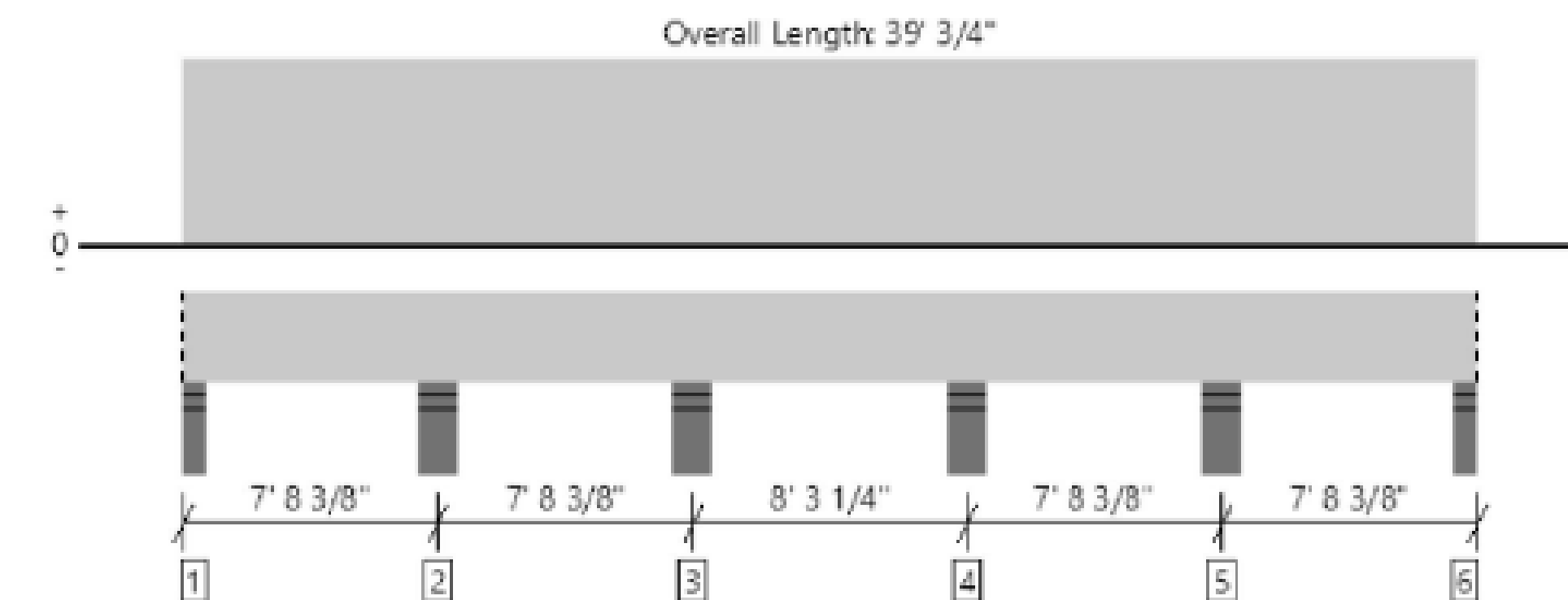
NEW CONSTRUCTION
2869 4th Street
Atlanta, GA 30315



DATE:	3/16/2023
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SHEET:	D2

Proposed First Floor Framing Plan			
Member Name	Results	Current Solution	Comments
Beam "A"	Passed	3 piece(s) 2 x 12 SPF No.1/No.2	
Beam "B"	Passed	2 piece(s) 2 x 10 SPF No.1/No.2	
Joist "A"	Passed	1 piece(s) 2 x 12 SPF No.1/No.2 @ 16" OC	
Joist "B"	Passed	1 piece(s) 2 x 8 SPF No.1/No.2 @ 16" OC	
Proposed Second Floor Framing Plan			
Member Name	Results	Current Solution	Comments
Beam "C"	Passed	4 piece(s) 1 3/4" x 16" 2.0E MicroLam@ LVL	
Joist "C"	Passed	1 piece(s) 2 x 12 SPF No.1/No.2 @ 16" OC	
Proposed Second Floor Ceiling Framing Plan			
Member Name	Results	Current Solution	Comments
Joist "D"	Passed	1 piece(s) 2 x 8 SPF No.1/No.2 @ 16" OC	
Proposed Roof Framing Plan			
Member Name	Results	Current Solution	Comments
Ridge Beam "D"	Passed	2 piece(s) 1 3/4" x 11 7/8" 2.0E MicroLam@ LVL	
Ridge Beam "E"	Passed	1 piece(s) 1 3/4" x 11 7/8" 2.0E MicroLam@ LVL	
Valley Beam "F"	Passed	1 piece(s) 1 3/4" x 11 7/8" 2.0E MicroLam@ LVL	
Rafter "A"	Passed	1 piece(s) 2 x 8 SPF No.1/No.2 @ 16" OC	
Rafter "B"	Passed	1 piece(s) 2 x 6 SPF No.1/No.2 @ 16" OC	

Proposed First Floor Framing Plan, Beam "A"
3 piece(s) 2 x 12 SPF No.1/No.2



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	8223 @ 7' 8 3/8"	11475 (6,00")	Passed (72%)	-	1.0 D + 1.0 L (Adj Spans)
Shear (lbs)	3153 @ 6' 6 1/8"	4556	Passed (69%)	1.00	1.0 D + 1.0 L (Adj Spans)
Moment (Ft-lbs)	-6018 @ 7' 8 3/8"	6921	Passed (87%)	1.00	1.0 D + 1.0 L (Adj Spans)
Live Load Defl. (in)	0,045 @ 3' 9 9/16"	0,188	Passed (L/999+)	-	1.0 D + 1.0 L (Alt Spans)
Total Load Defl. (in)	0,060 @ 3' 8 11/16"	0,377	Passed (L/999+)	-	1.0 D + 1.0 L (Alt Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- Applicable calculations are based on NDS.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Factored	
1 - Stud wall - SPF	3,50"	3,50"	1,63"	984	2141/253	3125	Blocking
2 - Stud wall - SPF	6,00"	6,00"	4,30"	2658	5565	8223	None
3 - Stud wall - SPF	6,00"	6,00"	4,20"	2469	5561	8030	None
4 - Stud wall - SPF	6,00"	6,00"	4,20"	2469	5561	8030	None
5 - Stud wall - SPF	6,00"	6,00"	4,30"	2657	5564	8221	None
6 - Stud wall - SPF	3,50"	3,50"	1,63"	984	2140/253	3124	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	16' 11" o/c	
Bottom Edge (Lu)	11' 1" o/c	

• Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0,90)	Floor Live (1,00)	Comments
0 - Self Weight (PLF)	0 to 39' 3/4"	N/A	12,8	-	
1 - Uniform (PSF)	0 to 39' 3/4" (Front)	15'	20,0	40,0	Default Load

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The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

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NEW CONSTRUCTION
2869 4th Street
Atlanta, GA 30315



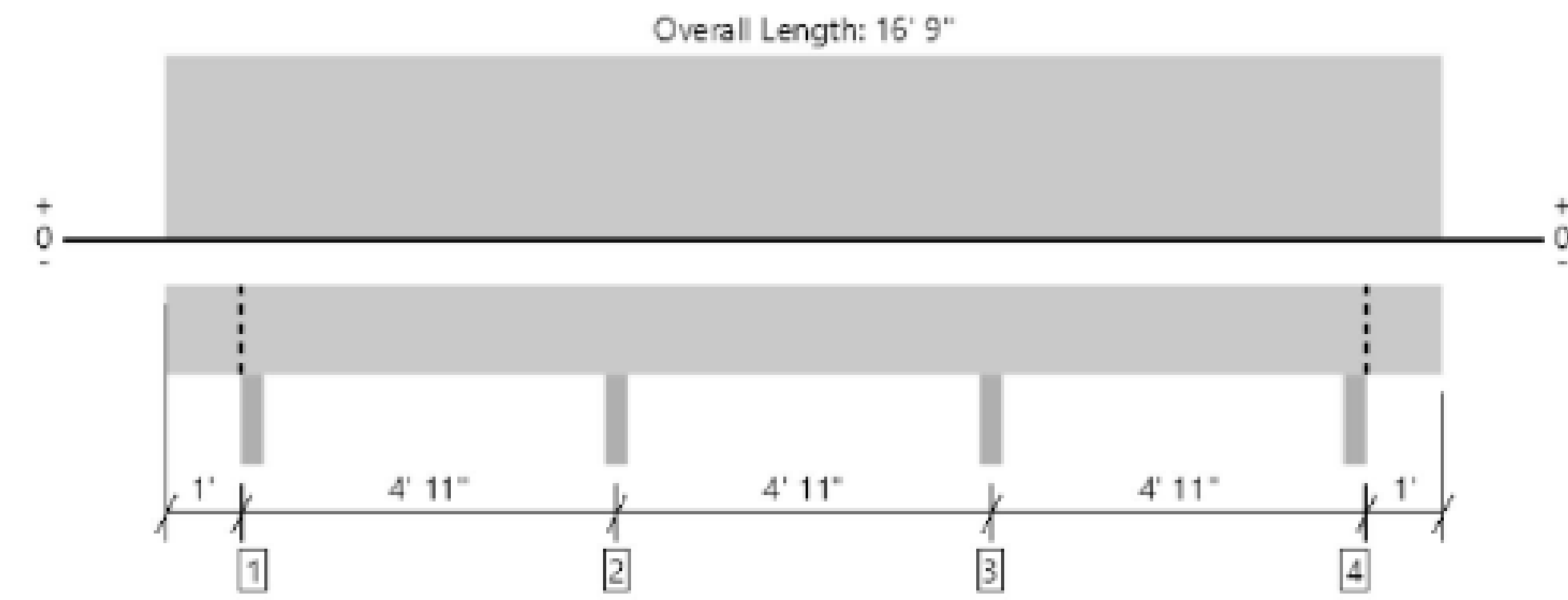
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SHEET:
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Proposed First Floor Framing Plan, Beam "B"
2 piece(s) 2 x 10 SPF No.1/No.2



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1723 @ 5' 11"	4463 (3,50")	Passed (39%)	-	1.0 D + 1.0 L (Adj Spans)
Shear (lbs)	602 @ 5'	2498	Passed (24%)	1.00	1.0 D + 1.0 L (Adj Spans)
Moment (Ft-lbs)	-790 @ 10' 10"	3431	Passed (23%)	1.00	1.0 D + 1.0 L (Adj Spans)
Live Load Defl. (in)	0.006 @ 13' 3 7/16"	0.119	Passed (L/999+)	-	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.008 @ 3' 5 5/16"	0.239	Passed (L/999+)	-	1.0 D + 1.0 L (All Spans)

- Deflection criteria: LL (L/480) and TL (L/240).
- Overhang deflection criteria: LL (2L/480) and TL (2L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- Applicable calculations are based on NDS.

System: Floor
Member Type: Flush Beam
Building Use: Residential
Building Code: IBC 2018
Design Methodology: ASD

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Factored	
1 - Column - SPF	3,50"	3,50"	1,50"	343	694	1037	Blocking
2 - Column - SPF	3,50"	3,50"	1,50"	553	1170	1723	None
3 - Column - SPF	3,50"	3,50"	1,50"	553	1170	1723	None
4 - Column - SPF	3,50"	3,50"	1,50"	343	694	1037	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	16' 9" o/c	
Bottom Edge (Lu)	16' 9" o/c	

• Maximum allowable bracing intervals based on applied load.

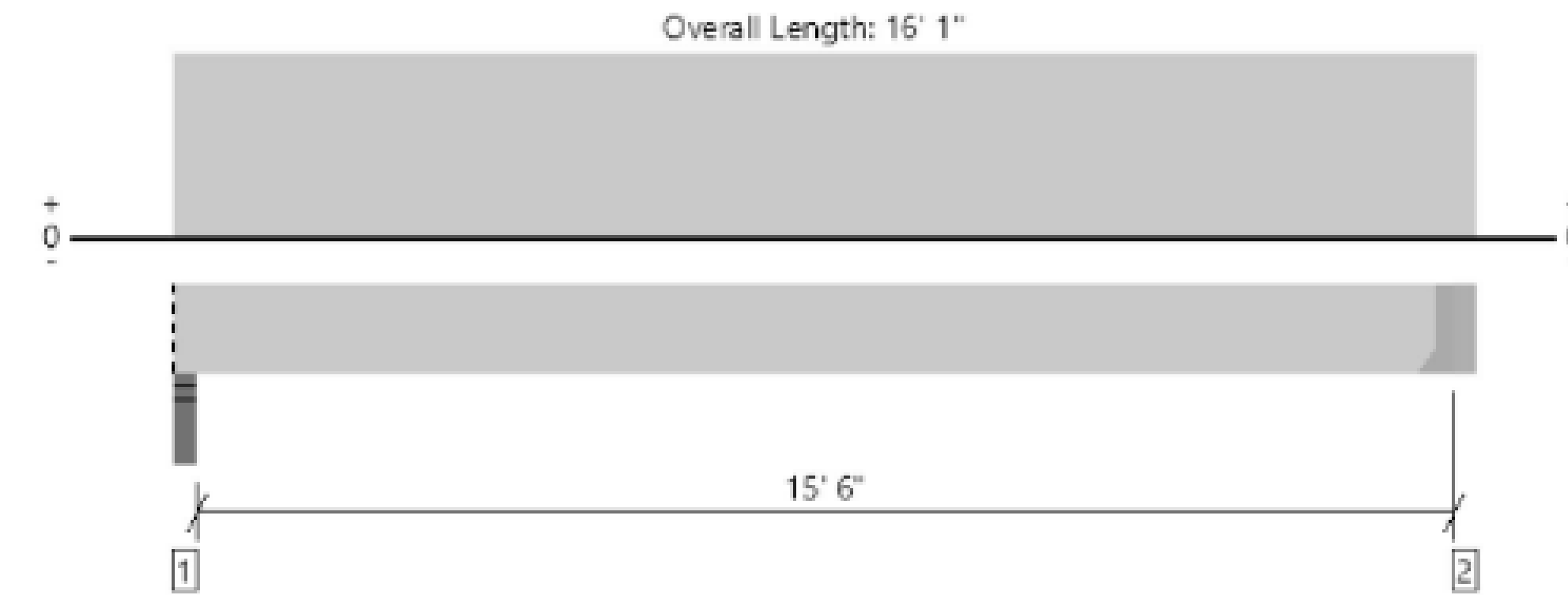
Vertical Loads	Location (Side)	Tributary Width	Dead (0,90)	Floor Live (1,00)	Comments
0 - Self Weight (PLF)	0 to 16' 9"	N/A	7,0	-	
1 - Uniform (PSF)	0 to 16' 9" (Front)	5'	20,0	40,0	Default Load

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The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

Proposed First Floor Framing Plan, Joist "A"
1 piece(s) 2 x 12 SPF No.1/No.2 @ 16" OC



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	623 @ 15' 9 1/2"	956 (1,50")	Passed (65%)	-	1.0 D + 1.0 L (All Spans)
Shear (lbs)	548 @ 14' 10 1/4"	1519	Passed (36%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	2428 @ 8'	2653	Passed (92%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0,284 @ 8'	0,390	Passed (L/658)	-	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0,426 @ 8'	0,779	Passed (L/439)	-	1.0 D + 1.0 L (All Spans)
T-Pro™ Rating	N/A	N/A	N/A	-	N/A

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- A 15% increase in the moment capacity has been added to account for repetitive member usage.
- Applicable calculations are based on NDS.
- No composite action between deck and joist was considered in analysis.

System: Floor
Member Type: Joist
Building Use: Residential
Building Code: IBC 2018
Design Methodology: ASD

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Factored	
1 - Stud wall - SPF	3,50"	3,50"	1,50"	213	427	640	Blocking
2 - Hanger on 11 1/4" SPF beam	3,50"	Hanger ¹	1,50"	216	431	647	See note ¹

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- ¹ See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	3' 6" o/c	
Bottom Edge (Lu)	15' 10" o/c	

• Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie

Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
2 - Face Mount Hanger	LUS28	1,75"	N/A	6-10d x 1.5	3-10d	

• Refer to manufacturer notes and instructions for proper installation and use of all connectors.

Vertical Load	Location (Side)	Spacing	Dead (0,90)	Floor Live (1,00)	Comments
1 - Uniform (PSF)	0 to 16' 1"	16"	20,0	40,0	Default Load

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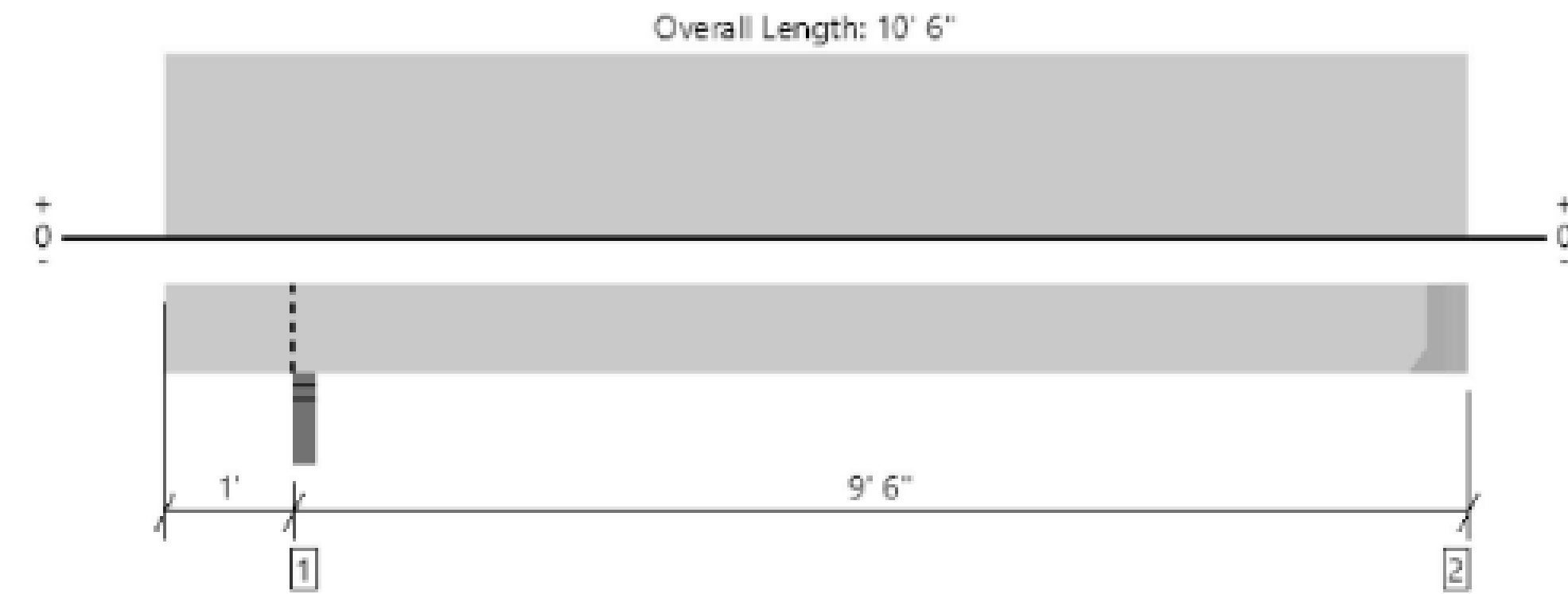
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Proposed First Floor Framing Plan, Joist "B"
1 piece(s) 2 x 8 SPF No.1/No.2 @ 16" OC



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDf	Load: Combination (Pattern)
Member Reaction (lbs)	361 @ 10' 2 1/2"	956 (1.50")	Passed (38%)	—	1.0 D + 1.0 L (All Spans)
Shear (lbs)	312 @ 9' 7 1/4"	979	Passed (32%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	813 @ 5' 8 7/16"	1322	Passed (61%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.121 @ 5' 8 1/8"	0.227	Passed (L/896)	—	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.180 @ 5' 8 1/4"	0.453	Passed (L/605)	—	1.0 D + 1.0 L (All Spans)
TJ-Pro™ Rating	N/A	N/A	N/A	—	N/A

System : Floor
Member Type : Joist
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Overhang deflection criteria: LL (2L/480) and TL (2L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- A 15% increase in the moment capacity has been added to account for repetitive member usage.
- Applicable calculations are based on NDS.
- No composite action between deck and joist was considered in analysis.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Factored	
1 - Stud wall - SPF	3.50"	3.50"	1.50"	153	307	460	Blocking
2 - Hanger on 7 1/4" SPF beam	3.50"	Hanger ¹	1.50"	127	257	384	See note ¹

- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.
- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- ¹ See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	10' 3" o/c	
Bottom Edge (Lu)	10' 3" o/c	

• Maximum allowable bracing intervals based on applied load.

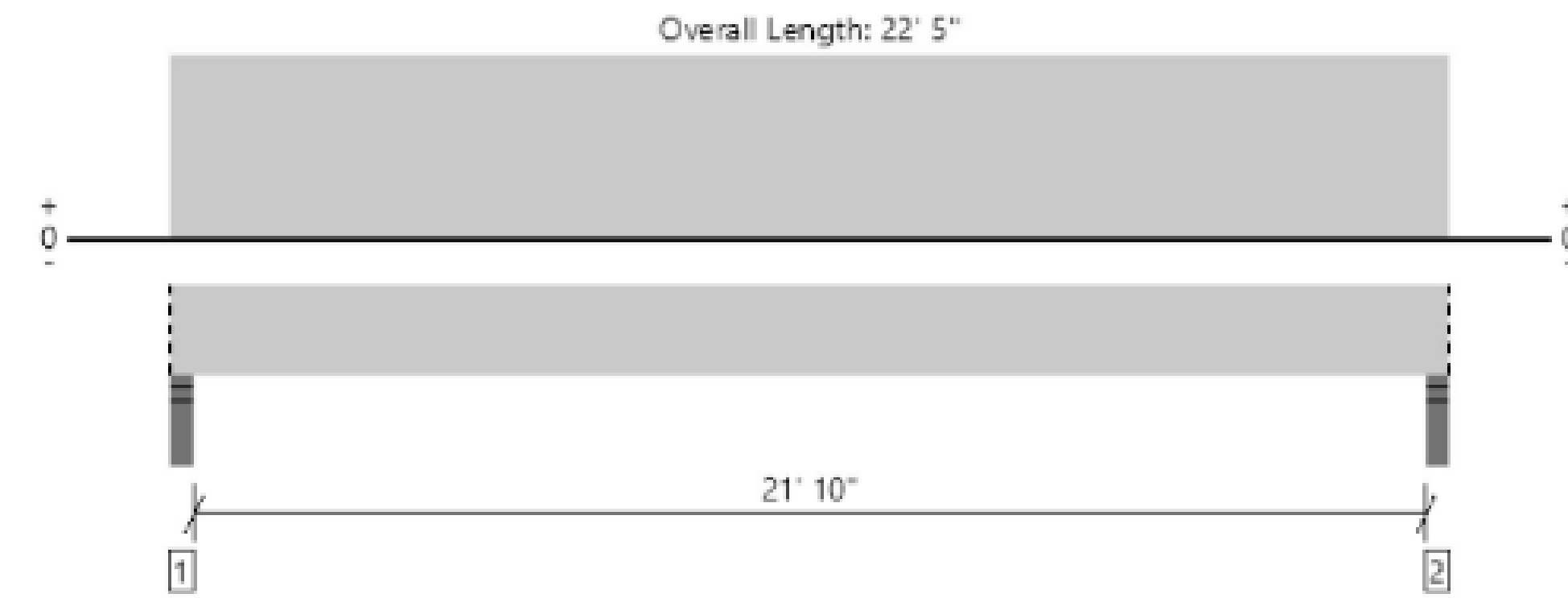
Connector: Simpson Strong-Tie						
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
2 - Face Mount Hanger	LU26	1.50"	N/A	6-10dx1.5	4-10dx1.5	

- Refer to manufacturer notes and instructions for proper installation and use of all connectors.

Vertical Load	Location (Side)	Spacing	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform (PSF)	0 to 10' 6"	16"	20.0	40.0	Default Load

Weyerhaeuser Notes

Proposed Second Floor Framing Plan, Beam "C"
4 piece(s) 1 3/4" x 16" 2.0E Microllam® LVL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDf	Load: Combination (Pattern)
Member Reaction (lbs)	10286 @ 2"	10413 (3.50")	Passed (99%)	—	1.0 D + 1.0 L (All Spans)
Shear (lbs)	8794 @ 1' 7 1/2"	21280	Passed (41%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	55940 @ 11' 2 1/2"	62228	Passed (90%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.698 @ 11' 2 1/2"	0.736	Passed (L/380)	—	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	1.085 @ 11' 2 1/2"	1.104	Passed (L/244)	—	1.0 D + 1.0 L (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- Member should be side-loaded from both sides of the member or braced to prevent rotation.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Factored	
1 - Stud wall - SPF	3.50"	3.50"	3.46"	3673	6613	10286	Blocking
2 - Stud wall - SPF	3.50"	3.50"	3.46"	3673	6613	10286	Blocking

- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	7' 1" o/c	
Bottom Edge (Lu)	22' 5" o/c	

• Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Comments
0 - Self Weight (PLF)	0 to 22' 5"	N/A	32.7	—	
1 - Uniform (PSF)	0 to 22' 5" (Front)	14' 9"	20.0	40.0	Default Load

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The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

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2869 4th Street
Atlanta, GA 30315



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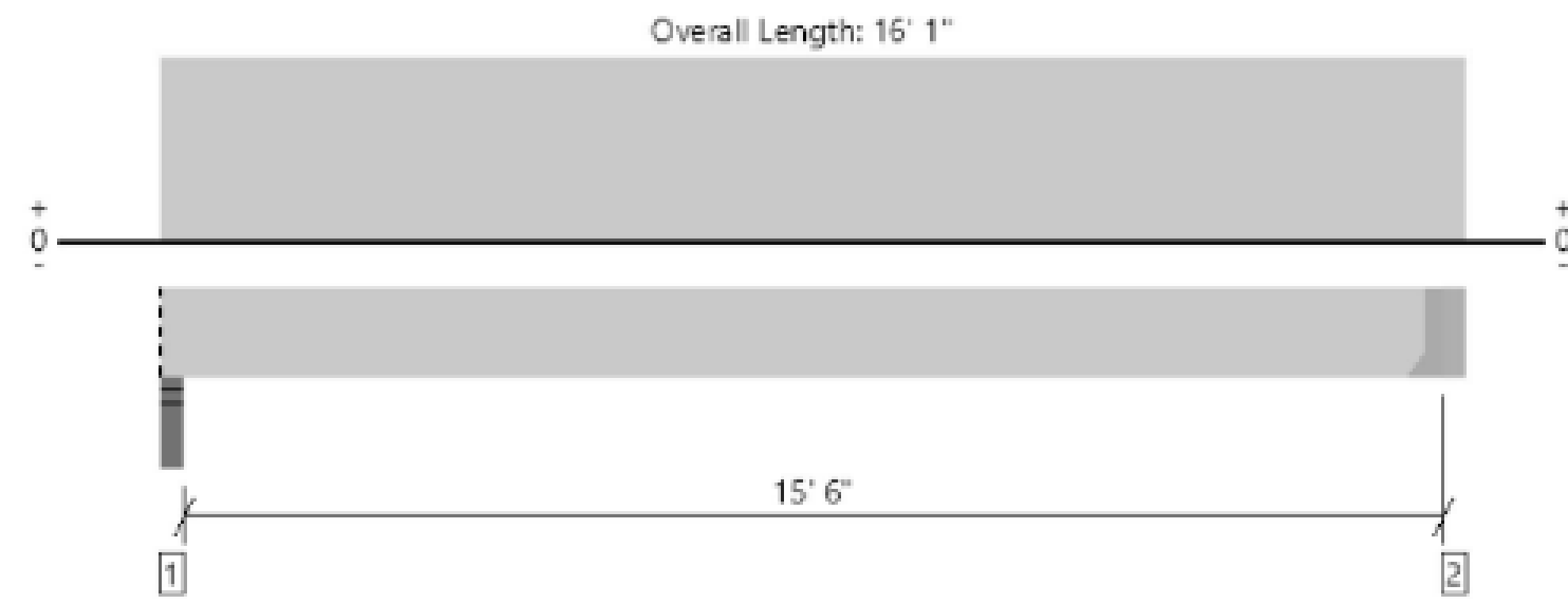
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Proposed Second Floor Framing Plan, Joist "C"
1 piece(s) 2 x 12 SPF No.1/No.2 @ 16" OC



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	623 @ 15' 9 1/2"	956 (1.50")	Passed (65%)	-	1.0 D + 1.0 L (All Spans)
Shear (lbs)	548 @ 14' 10 1/4"	1519	Passed (36%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	2428 @ 8'	2653	Passed (92%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.284 @ 8'	0.390	Passed (L/658)	-	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.426 @ 8'	0.779	Passed (L/439)	-	1.0 D + 1.0 L (All Spans)
T-Pro™ Rating	N/A	N/A	N/A	-	N/A

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- A 15% increase in the moment capacity has been added to account for repetitive member usage.
- Applicable calculations are based on NDS.
- No composite action between deck and joist was considered in analysis.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Factored	
1 - Stud wall - SPF	3.50"	3.50"	1.50"	213	427	640	Blocking
2 - Hanger on 11 1/4" LVL beam	3.50"	Hanger [†]	1.50"	216	431	647	See note [†]

- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.
- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger.
- [†] See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	3' 6" o/c	
Bottom Edge (Lu)	15' 10" o/c	

Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie						
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
2 - Face Mount Hanger	LUS28	1.75"	N/A	6-10dx1.5	3-10d	

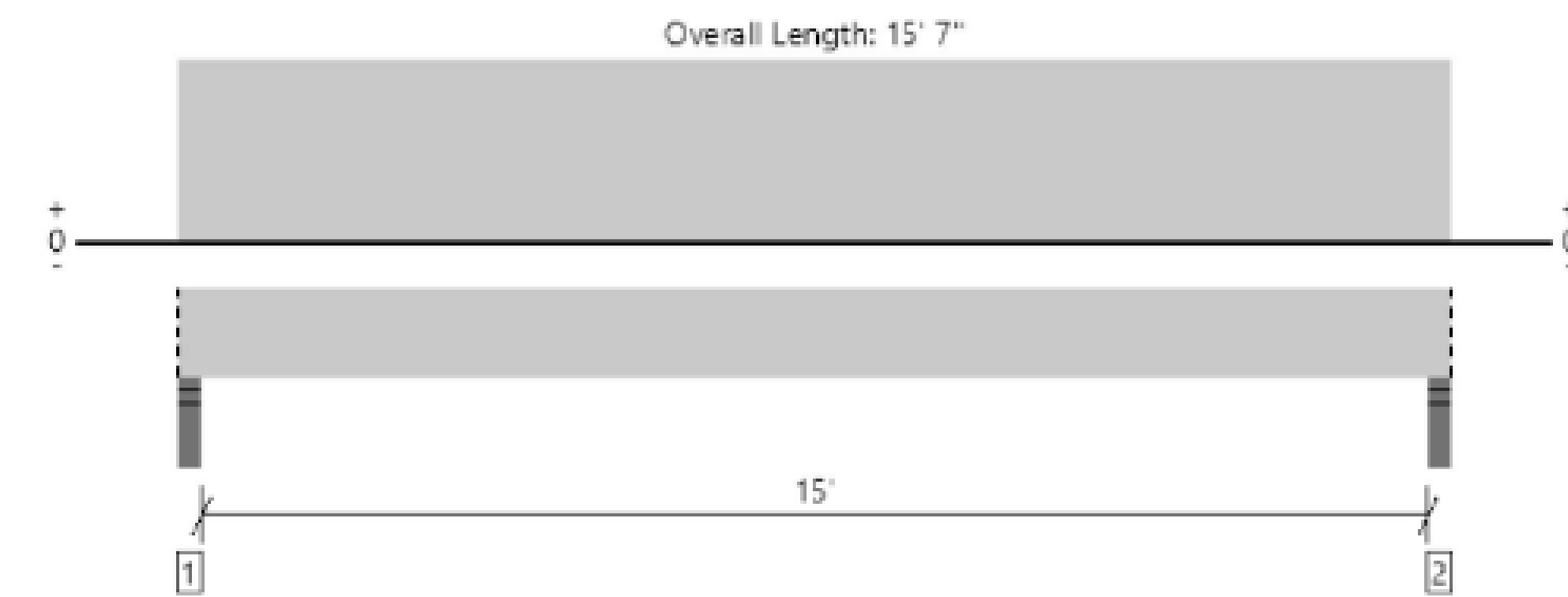
- Refer to manufacturer notes and instructions for proper installation and use of all connectors.

Vertical Load	Location (Side)	Spacing	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform (PSF)	0 to 16' 1"	16"	20.0	40.0	Default Load

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System : Floor
Member Type : Joist
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

Proposed Second Floor Ceiling Framing Plan, Joist "D"
1 piece(s) 2 x 8 SPF No.1/No.2 @ 16" OC



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	312 @ 2 1/2"	2231 (3.50")	Passed (14%)	-	1.0 D + 1.0 Lr (All Spans)
Shear (lbs)	276 @ 10 3/4"	1223	Passed (23%)	1.25	1.0 D + 1.0 Lr (All Spans)
Moment (Ft-lbs)	1150 @ 7' 9 1/2"	1653	Passed (70%)	1.25	1.0 D + 1.0 Lr (All Spans)
Live Load Defl. (in)	0.476 @ 7' 9 1/2"	0.758	Passed (L/382)	-	1.0 D + 1.0 Lr (All Spans)
Total Load Defl. (in)	0.714 @ 7' 9 1/2"	1.011	Passed (L/255)	-	1.0 D + 1.0 Lr (All Spans)

- Deflection criteria: LL (L/240) and TL (L/180).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- A 15% increase in the moment capacity has been added to account for repetitive member usage.
- Applicable calculations are based on NDS.

System : Roof
Member Type : Joist
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD
Member Pitch : 0/12

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Roof Live	Factored	
1 - Stud wall - SPF	3.50"	3.50"	1.50"	104	208	312	Blocking
2 - Stud wall - SPF	3.50"	3.50"	1.50"	104	208	312	Blocking

- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	7' 2" o/c	
Bottom Edge (Lu)	15' 7" o/c	

Maximum allowable bracing intervals based on applied load.

Vertical Load	Location (Side)	Spacing	Dead (0.90)	Roof Live (non-snow: 1.25)	Comments
1 - Uniform (PSF)	0 to 15' 7"	16"	10.0	20.0	Default Load

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The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

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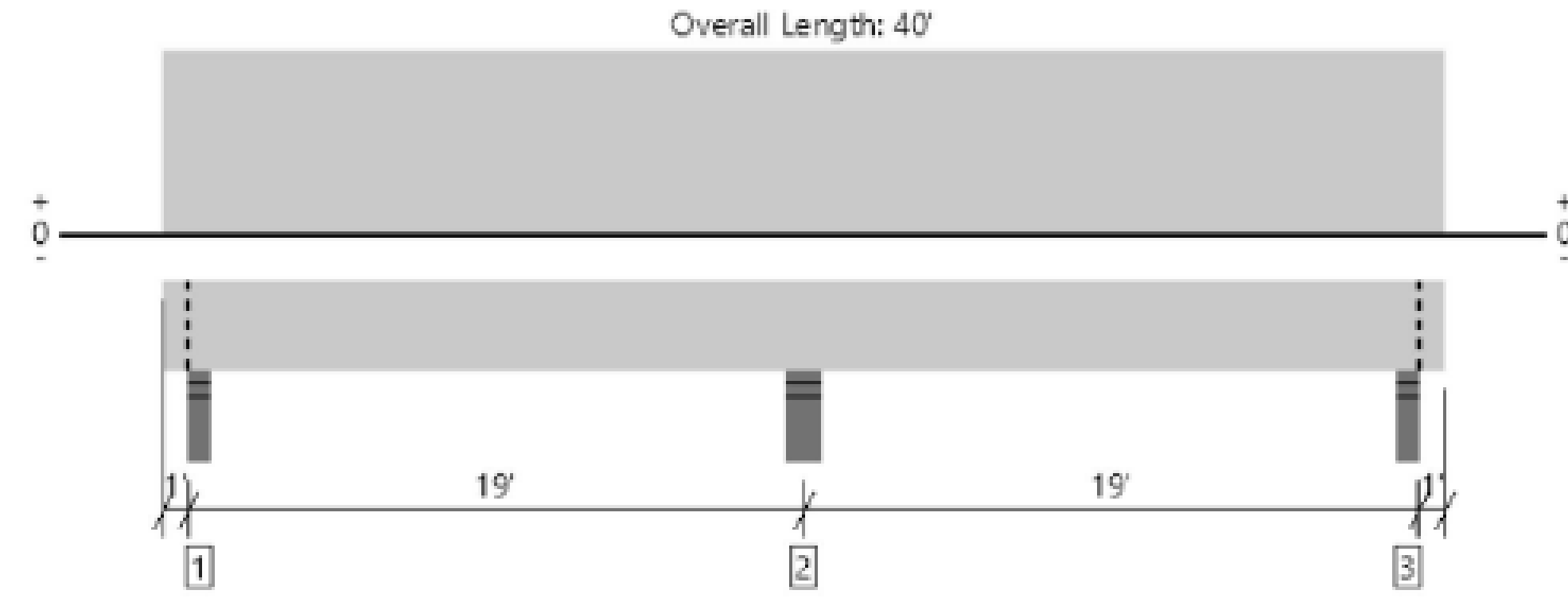
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Proposed Roof Framing Plan, Ridge Beam "D"
2 piece(s) 1 3/4" x 11 7/8" 2.0E Microllam® LVL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDf	Load: Combination (Pattern)
Member Reaction (lbs)	7334 @ 20'	7809 (5.25")	Passed (94%)	-	1.0 D + 1.0 Lr (Adj Spans)
Shear (lbs)	3290 @ 21' 2 1/2"	9871	Passed (33%)	1.25	1.0 D + 1.0 Lr (Adj Spans)
Moment (Ft-lbs)	-13800 @ 20'	22310	Passed (62%)	1.25	1.0 D + 1.0 Lr (Adj Spans)
Live Load Defl. (in)	0.348 @ 30' 3 7/8"	0.943	Passed (L/650)	-	1.0 D + 1.0 Lr (Alt Spans)
Total Load Defl. (in)	0.495 @ 30' 5 15/16"	1.257	Passed (L/457)	-	1.0 D + 1.0 Lr (Alt Spans)

System : Roof
 Member Type : Flush Beam
 Building Use : Residential
 Building Code : IBC 2018
 Design Methodology : ASD
 Member Pitch : 0/12

- Deflection criteria: LL (L/240) and TL (L/180).
- Overhang deflection criteria: LL (2L/240) and TL (2L/180).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Roof Live	Factored	
1 - Stud wall - SPF	3.50'	3.50'	1.81"	927	1772	2699	Blocking
2 - Stud wall - SPF	5.25'	5.25'	4.93"	2631	4703	7334	None
3 - Stud wall - SPF	3.50'	3.50'	1.81"	927	1772	2699	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	17' 6" o/c	
Bottom Edge (Lu)	9' 11" o/c	

• Maximum allowable bracing intervals based on applied load.

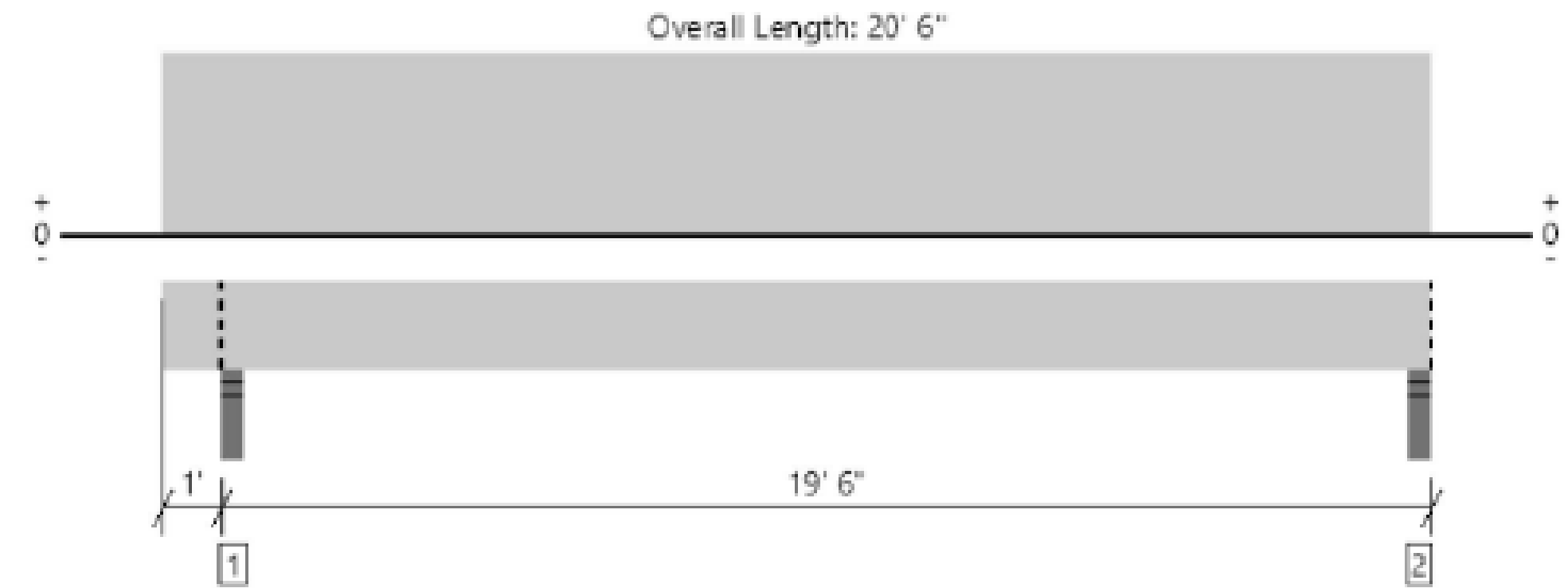
Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Comments
0 - Self Weight (PLF)	0 to 40'	N/A	12.1	-	
1 - Uniform (PSF)	0 to 40' (Front)	10'	10.0	20.0	Default Load

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Proposed Roof Framing Plan, Ridge Beam "E"
1 piece(s) 1 3/4" x 11 7/8" 2.0E Microllam® LVL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDf	Load: Combination (Pattern)
Member Reaction (lbs)	1681 @ 1' 1 3/4"	2603 (3.50")	Passed (65%)	-	1.0 D + 1.0 Lr (Alt Spans)
Shear (lbs)	1325 @ 2' 3 3/8"	4936	Passed (27%)	1.25	1.0 D + 1.0 Lr (Alt Spans)
Moment (Ft-lbs)	7147 @ 10' 9 1/8"	11155	Passed (64%)	1.25	1.0 D + 1.0 Lr (Alt Spans)
Live Load Defl. (in)	0.647 @ 10' 8 15/16"	0.959	Passed (L/356)	-	1.0 D + 1.0 Lr (Alt Spans)
Total Load Defl. (in)	1.009 @ 10' 8 15/16"	1.279	Passed (L/228)	-	1.0 D + 1.0 Lr (Alt Spans)

System : Roof
 Member Type : Flush Beam
 Building Use : Residential
 Building Code : IBC 2018
 Design Methodology : ASD
 Member Pitch : 0/12

- Deflection criteria: LL (L/240) and TL (L/180).
- Overhang deflection criteria: LL (2L/240) and TL (2L/180).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Roof Live	Factored	
1 - Stud wall - SPF	3.50'	3.50'	2.26"	604	1077	1681	Blocking
2 - Stud wall - SPF	3.50'	3.50'	2.04"	545	974	1520	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	5' 7" o/c	
Bottom Edge (Lu)	20' 6" o/c	

• Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Comments
0 - Self Weight (PLF)	0 to 20' 6"	N/A	6.1	-	
1 - Uniform (PSF)	0 to 20' 6" (Front)	5'	10.0	20.0	Default Load

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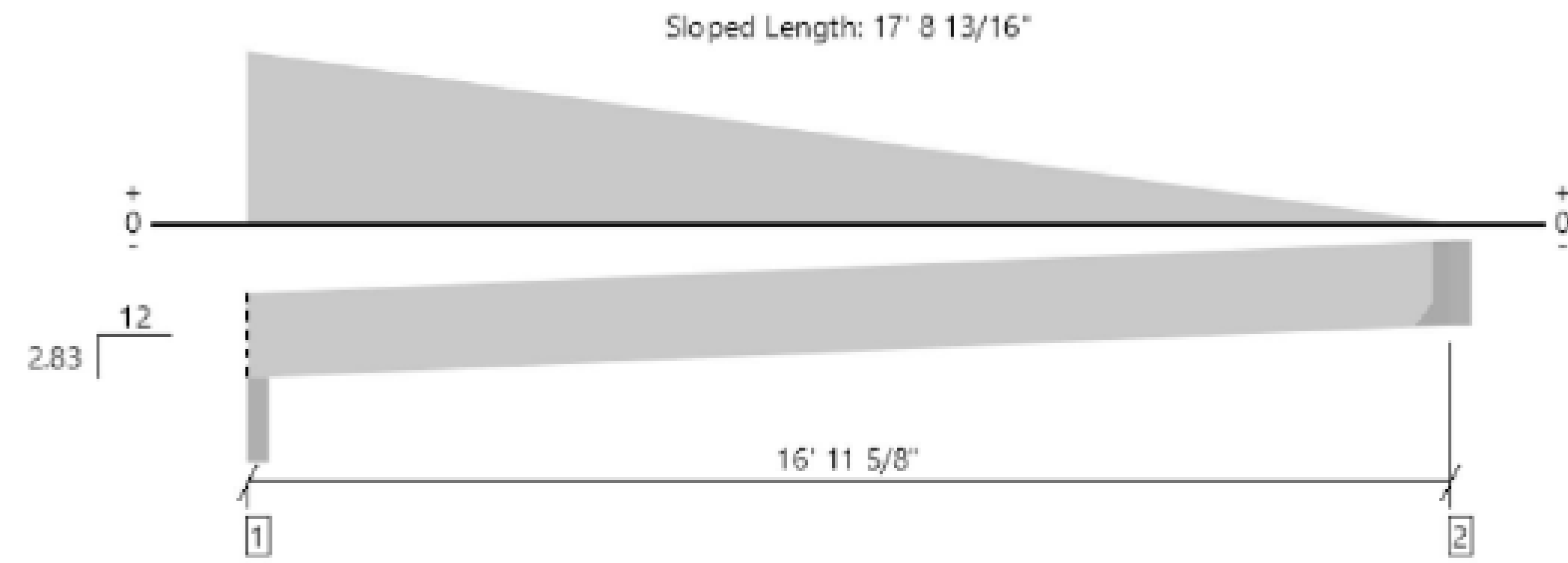
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3/16/2023

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Proposed Roof Framing Plan, Valley Beam "F"
1 piece(s) 1 3/4" x 11 7/8" 2.0E MicroIam® LVL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Member Length : 17' 8 1/16"

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1534 @ 2"	2603 (3,50")	Passed (59%)	-	1.0 D + 1.0 Lr (All Spans)
Shear (lbs)	1213 @ 1' 3 1/16"	4936	Passed (25%)	1.25	1.0 D + 1.0 Lr (All Spans)
Moment (Ft-lbs)	4863 @ 7' 3 7/8"	11155	Passed (44%)	1.25	1.0 D + 1.0 Lr (All Spans)
Live Load Defl. (in)	0.343 @ 8' 3"	0.863	Passed (L/604)	-	1.0 D + 1.0 Lr (All Spans)
Total Load Defl. (in)	0.549 @ 8' 3 1/8"	1.151	Passed (L/377)	-	1.0 D + 1.0 Lr (All Spans)

- Deflection criteria: LL (L/240) and TL (L/180).
- Allowed moment does not reflect the adjustment for the beam stability factor.

System : Roof
 Member Type : Flush Beam
 Building Use : Residential
 Building Code : IBC 2018
 Design Methodology : ASD
 Member Pitch : 2.83/12

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Roof Live	Factored	
1 - Beveled Plate - SPF	3.50"	3.50"	2.06"	564	969	1534	Blocking
2 - Hanger on 11 7/8" SPF beam	3.50"	Hanger ¹	1.50"	300	471	771	See note ¹

- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.
- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- ¹ See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	9' 1" o/c	
Bottom Edge (Lu)	17' 5" o/c	

•Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie

Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
2 - Face Mount Hanger	LSSR1.81Z	1.88"	N/A	14-10dx2.5	12-10dx1.5	

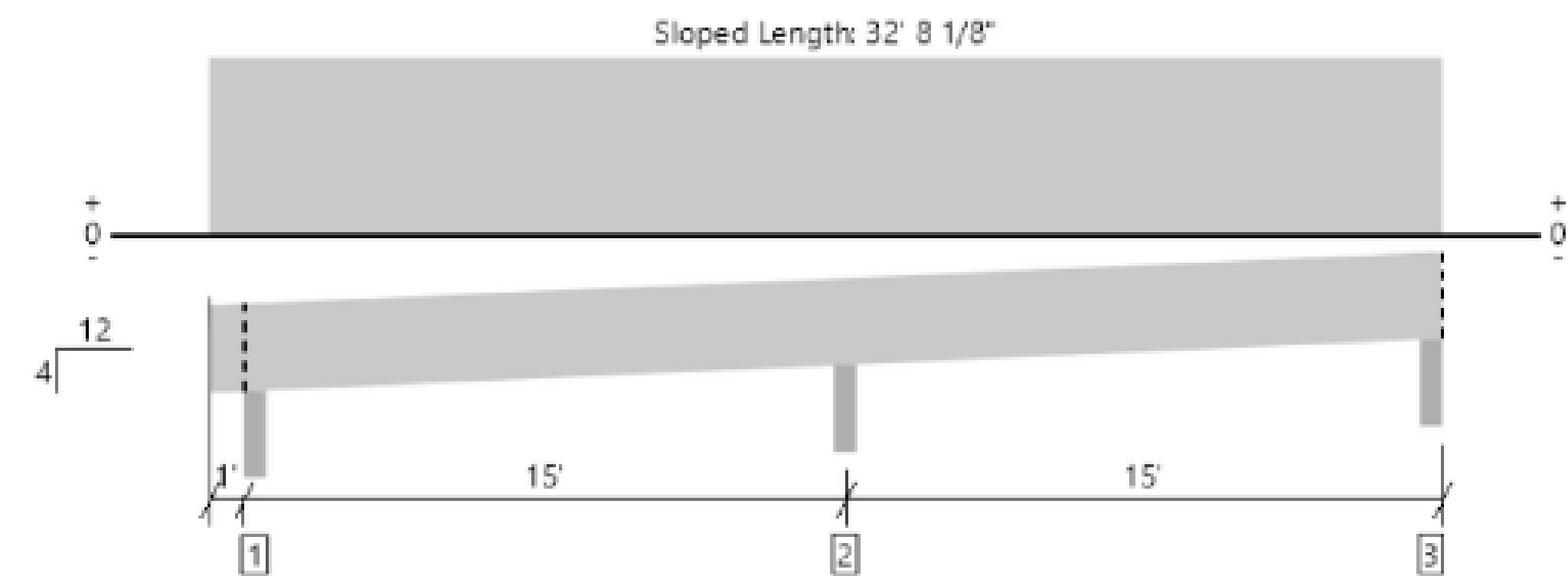
- Refer to manufacturer notes and instructions for proper installation and use of all connectors.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Comments
0 - Self Weight (PLF)	0 to 16' 11 5/8"	N/A	6.1	-	
1 - Tapered (PLF)	0 to 16' 11 5/8"	N/A	87.1 to 0.0	169.7 to 0.0	Generated from Roof Geometry

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Proposed Roof Framing Plan, Rafter "A"
1 piece(s) 2 x 8 SPF No.1/No.2 @ 16" OC



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Member Length : 32' 10 9/16"

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	753 @ 16'	2352 (3,50")	Passed (32%)	-	1.0 D + 1.0 Lr (Adj Spans)
Shear (lbs)	347 @ 16' 8 5/8"	1223	Passed (28%)	1.25	1.0 D + 1.0 Lr (Adj Spans)
Moment (Ft-lbs)	-1114 @ 16'	1653	Passed (67%)	1.25	1.0 D + 1.0 Lr (Adj Spans)
Live Load Defl. (in)	0.268 @ 24' 1 1/4"	0.780	Passed (L/698)	-	1.0 D + 1.0 Lr (All Spans)
Total Load Defl. (in)	0.373 @ 24' 2 15/16"	1.039	Passed (L/502)	-	1.0 D + 1.0 Lr (All Spans)

- Deflection criteria: LL (L/240) and TL (L/180).
- Overhang deflection criteria: LL (2L/240) and TL (2L/180).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- A 15% increase in the moment capacity has been added to account for repetitive member usage.
- Applicable calculations are based on NDS.

System : Roof
 Member Type : Joist
 Building Use : Residential
 Building Code : IBC 2018
 Design Methodology : ASD
 Member Pitch : 4/12

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Roof Live	Factored	
1 - Beveled Plate - SPF	3.50"	3.50"	1.50"	95	193	288	Blocking
2 - Beveled Plate - SPF	3.50"	3.50"	1.50"	259	493	753	None
3 - Beveled Plate - SPF	3.50"	3.50"	1.50"	81	166	247	Blocking

- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	13' o/c	
Bottom Edge (Lu)	7' 6" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Load	Location (Side)	Spacing	Dead (0.90)	Roof Live (non-snow: 1.25)	Comments
1 - Uniform (PSF)	0 to 31'	16"	10.0	20.0	Default Load

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Proposed Roof Framing Plan, Rafter "B"
1 piece(s) 2 x 6 SPF No.1/No.2 @ 16" OC



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Member Length : 9' 4 1/2"

System : Roof
 Member Type : Joist
 Building Use : Residential
 Building Code : IBC 2018
 Design Methodology : ASD
 Member Pitch : 4/12

Design Results	Actual @ Location	Allowed	Result	LDf	Load: Combination (Pattern)
Member Reaction (lbs)	201 @ 1' 1 3/4"	2352 (3,50")	Passed (9%)	-	1.0 D + 1.0 Lr (All Spans)
Shear (lbs)	131 @ 1' 8 11/16"	928	Passed (14%)	1.25	1.0 D + 1.0 Lr (All Spans)
Moment (Ft-lbs)	270 @ 4' 10 13/16"	1030	Passed (26%)	1.25	1.0 D + 1.0 Lr (All Spans)
Live Load Defl. (in)	0.067 @ 4' 10 5/16"	0.390	Passed (L/999+)	-	1.0 D + 1.0 Lr (All Spans)
Total Load Defl. (in)	0.101 @ 4' 10 3/8"	0.520	Passed (L/930)	-	1.0 D + 1.0 Lr (All Spans)

- Deflection criteria: LL (L/240) and TL (L/180).
- Overhang deflection criteria: LL (2L/240) and TL (2L/180).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- A 15% increase in the moment capacity has been added to account for repetitive member usage.
- Applicable calculations are based on NDS.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Roof Live	Factored	
1 - Beveled Plate - SPF	3.50'	3.50'	1.50"	69	132	201	Blocking
2 - Beveled Plate - SPF	3.50'	3.50'	1.50"	54	103	157	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	9' 3" o/c	
Bottom Edge (Lu)	9' 3" o/c	

• Maximum allowable bracing intervals based on applied load.

Vertical Load	Location (Side)	Spacing	Dead (0.90)	Roof Live (non-snow: 1.25)	Comments
1 - Uniform (PSF)	0 to 8' 9"	16"	10.0	20.0	Default Load

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